

ESTIMATE OF QUANTITIES

ITEM DESCRIPTION	TOTAL QUANTITIES
STRUCTURAL EARTH EXCAVATION-PIERS	675 C.Y.
STRUCTURAL ROCK EXCAVATION-PIERS	20 C.Y.
GRAVEL BORROW	23,650 C.Y.
PORTLAND CEMENT CONCRETE, ABUTMENTS & RETAINING WALLS	400 C.Y.
PORTLAND CEMENT CONCRETE, PIERS	520 C.Y.
PORTLAND CEMENT CONCRETE, PIERS (PLACED UNDER WATER)	750 C.Y.
PORTLAND CEMENT CONCRETE, ROADWAY & SIDEWALK ON STEEL BRIDGES	450 C.Y.
PORTLAND CEMENT	3,365 BBLs
STRUCTURAL STEEL, FABRICATED & DELIVERED	LUMP SUM
STRUCTURAL STEEL, ERECTION	LUMP SUM
STRUCTURAL STEEL, FIELD PAINTING	LUMP SUM
STEEL RAIL, ALTERNATE "B"	956 L.F.
REINFORCING STEEL, DELIVERED	203,000 LBS
REINFORCING STEEL PLACED	203,000 LBS
STEEL H-BEAM PILES 42 LBS./SQ.FT.	3,730 L.F.
STEEL H-BEAM PILES 73 LBS./SQ.FT.	1,500 L.F.
COFFERDAM PIER 1 NORTHBOUND	LUMP SUM
COFFERDAM PIER 2 NORTHBOUND	LUMP SUM
COFFERDAM PIER 1 SOUTHBOUND	LUMP SUM
COFFERDAM PIER 2 SOUTHBOUND	LUMP SUM
ALUMINUM RAIL, ALTERNATE "A"	956 L.F.
GRANITE BRIDGE CURB	988 L.F.
PLAIN RIPRAP	2,000 C.Y.
HAND LAID RIPRAP	1,340 C.Y.
EPOXY RESIN SURFACE SEALANT	190 SY.

*BITUMINOUS CONCRETE SURFACE COURSE, TYPE A 223 TONS
 *MEMBRANE WATERPROOFING 1,990 SY.
 ESTIMATED QUANTITY OF STRUCTURAL STEEL INCLUDING DRAINS = 575,600 LBS.
 *NOT A PART OF THIS CONTRACT.

INDEX OF SHEETS

SHEET NUMBER	TITLE
1	INDEX, QUANTITIES, GENERAL PLAN
2	SURVEY
3	FOUNDATION SURVEY
4 THRU 7	BORING DETAILS
8	ABUTMENT 1 NORTHBOUND
9	ABUTMENT 2 NORTHBOUND
10	ABUTMENT 1 SOUTHBOUND
11	ABUTMENT 2 SOUTHBOUND
12	PIER 1 NORTHBOUND
12A	PIER 2 NORTHBOUND
13	PIERS 1 & 2 SOUTHBOUND
14	STRUCTURAL STEEL NORTHBOUND & SOUTHBOUND
15	ARMORED JOINT & EXPANSION DAM
16 & 17	SUPERSTRUCTURE NORTHBOUND & SOUTHBOUND
18	REINFORCING STEEL
BD 101-62	STANDARD DETAIL - BEARING PEDESTALS
BD 102-62	STANDARD DETAIL - BRIDGE RAIL
BD 103-62	STANDARD DETAIL - BEAM SPLICES

SPECIFICATIONS

DESIGN: A.A.S.H.O. Standard Specifications for Highway Bridges, 1961, with Interim Specifications.
 CONTENT: State of Maine, State Highway Commission, Standard Specifications for Highways and Bridges, Revision of January 1956, and Supplemental Specifications.

LIVE LOADING

H20-S16-44 as modified for Interstate Highways.

ALLOWABLE STRESSES

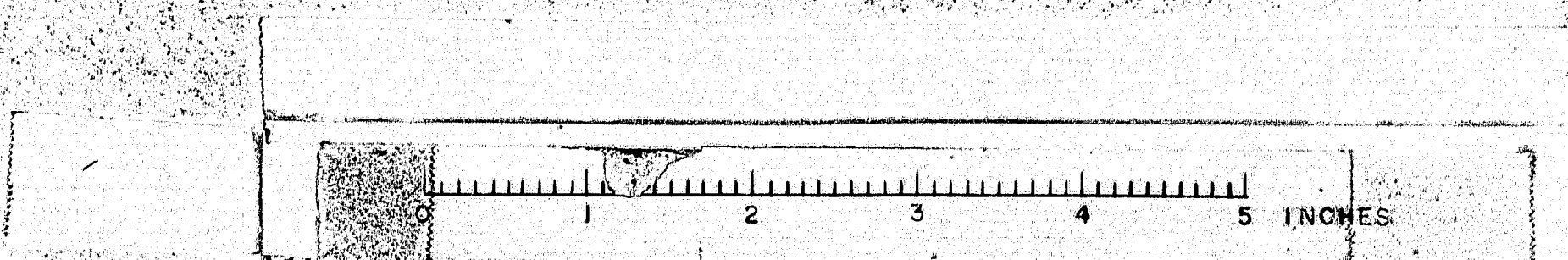
Concrete ($n=10$) ~ $f_c = 1200$ psi.
 Reinf. Steel, Int. Grap. ~ $f_s = 20,000$ psi.
 Structural Steel ~ $f_s = 20,000$ psi.

CONCRETE CLASSIFICATION

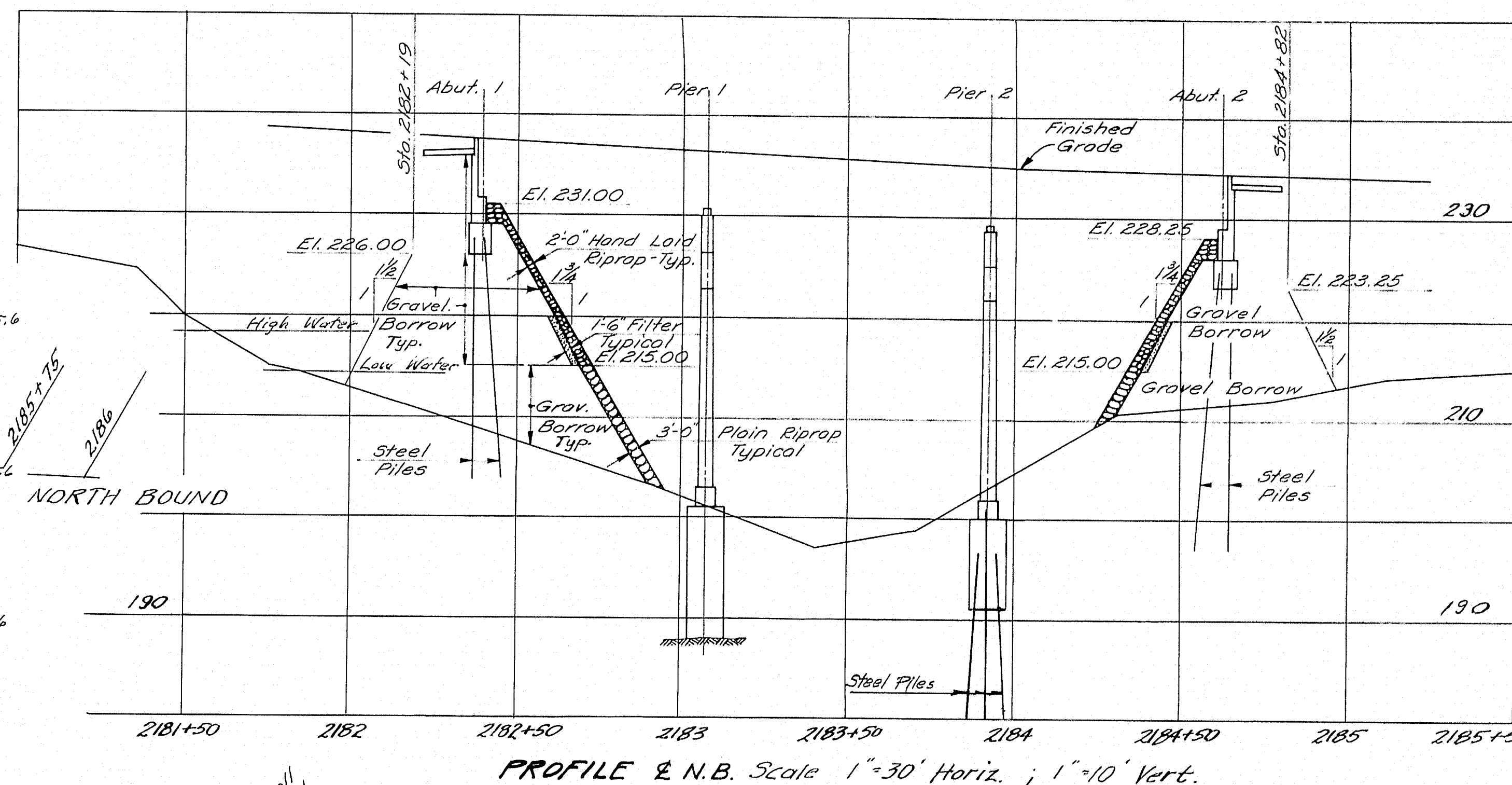
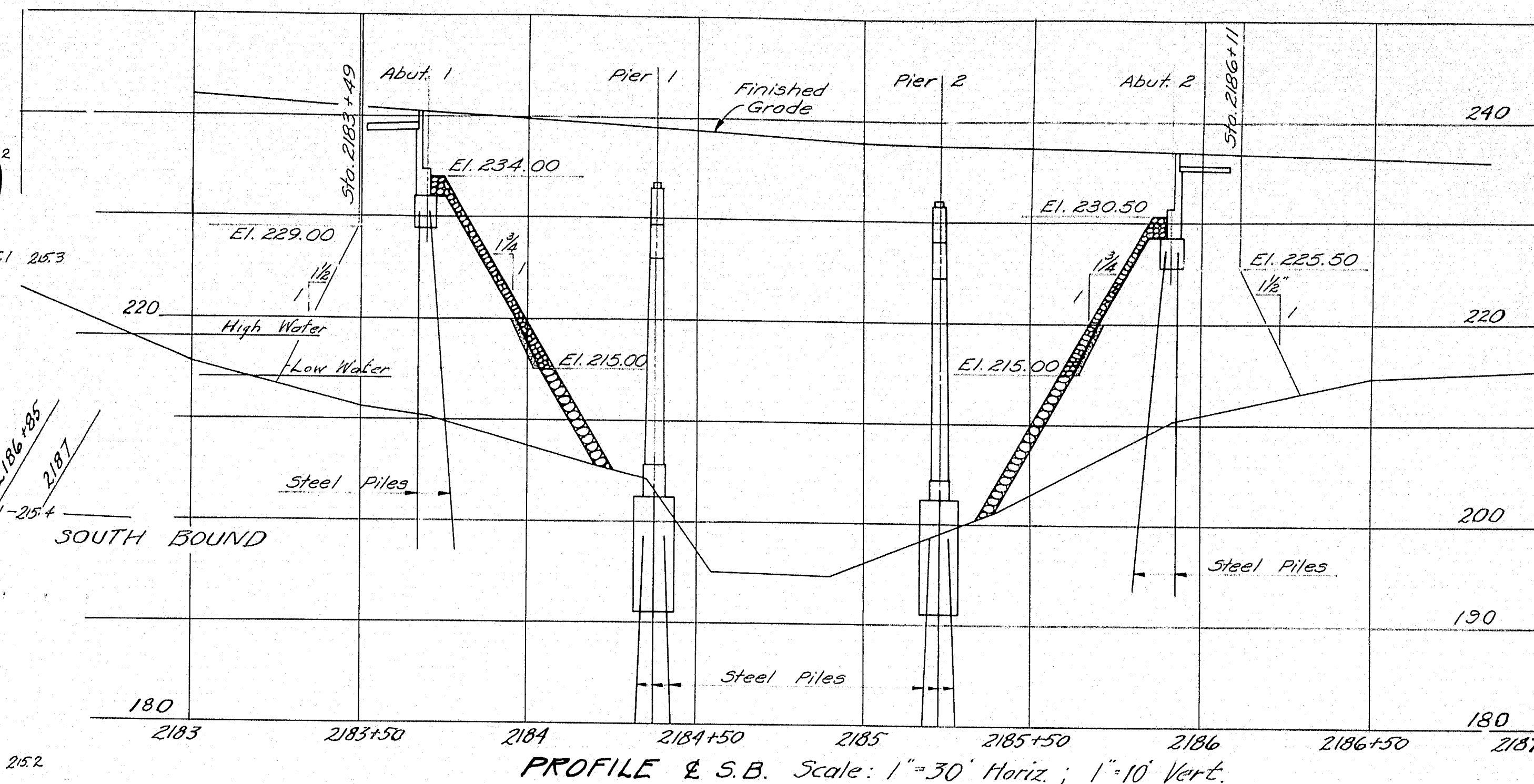
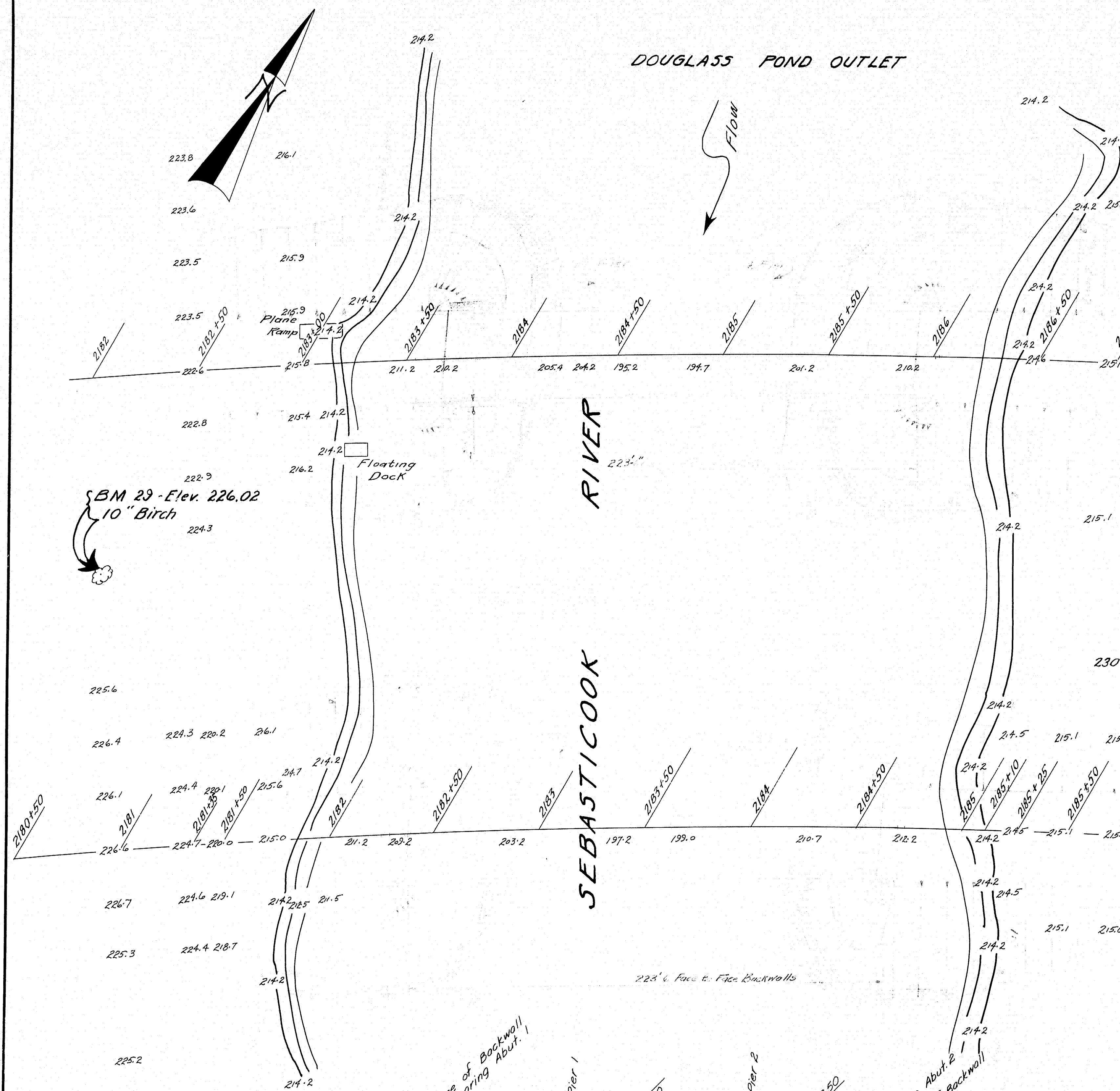
PIER SEALS: Class "3"
 ALL OTHER: Class "A"

DESIGN T.H.K. DETAIL E.E.L. CHECK	BRIDGE NO.
STATE HIGHWAY COMMISSION BRIDGE DIVISION	
SEBASTICOOK RIVER BRIDGE	
IN THE TOWN OF PITTSFIELD	
SOMERSET COUNTY	
GENERAL PLAN	
SHEET 1 OF 13 AUGUSTA, MAINE FEB. 1963	

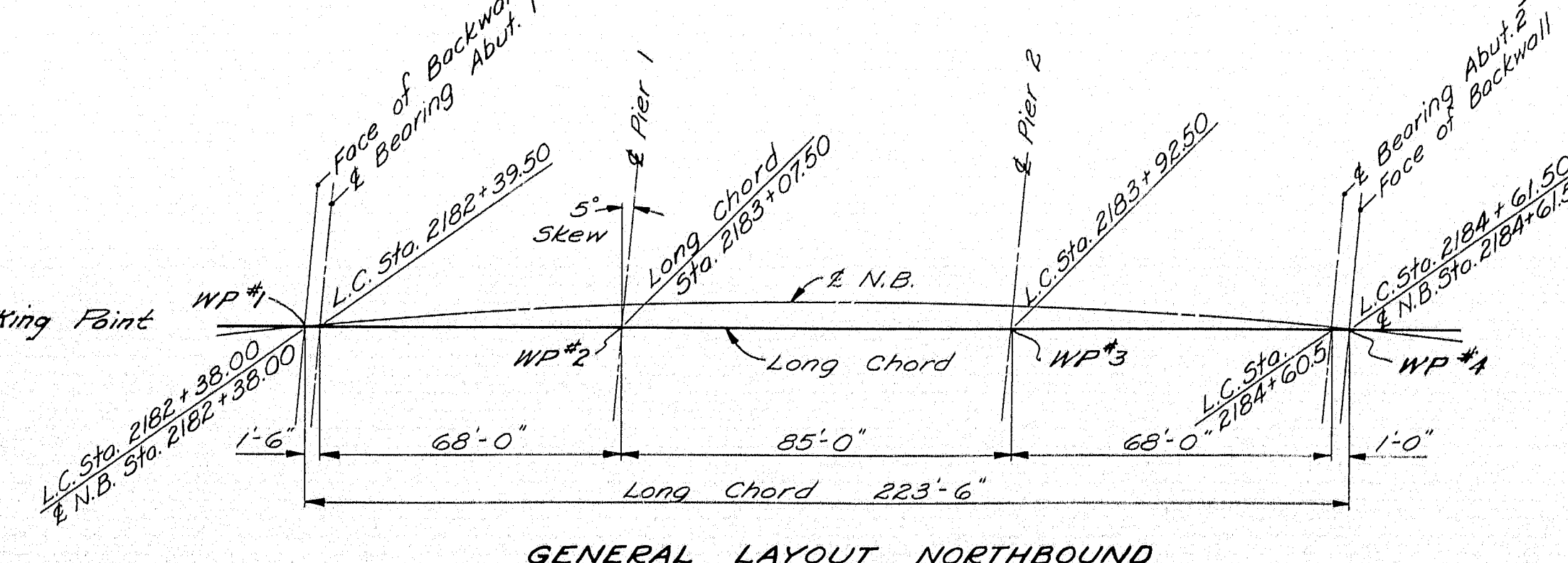
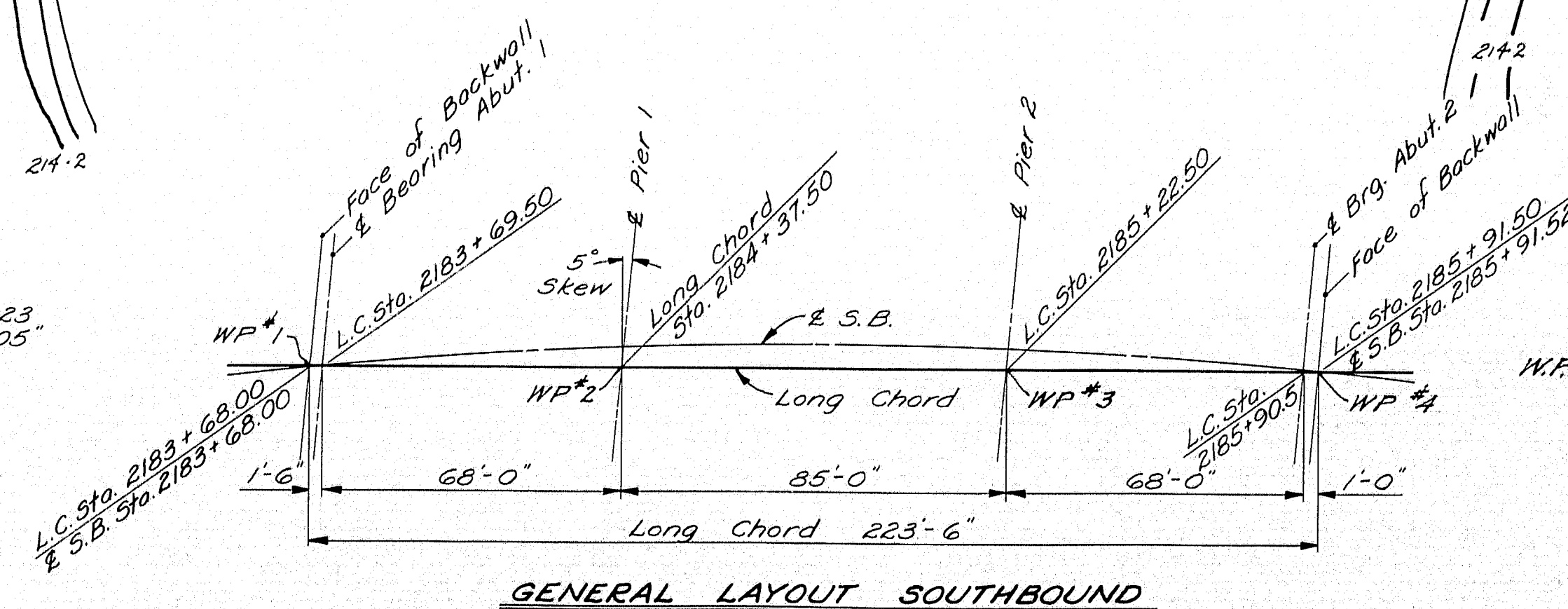
M-1980



NOTES
 STREAM - Water controlled by dam at Waverly Mills. Present stage, low water. Practically no water flowing over dam. High stage, 4.0' higher than present.
 UTILITIES - C.M.R. Co.

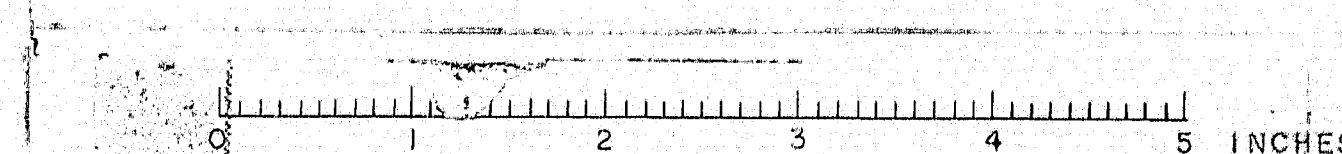


CURVE DATA	
S.B.	N.B.
P.I. 2178+78.36	P.I. 2178+08.23
Δ 53°-45'-05"	Δ 53°-45'-05"
D 1°-15'	D 1°-15'
T 2322.99	T 2322.99
L 4300.12	L 4300.12
R 4583.66	R 4583.66

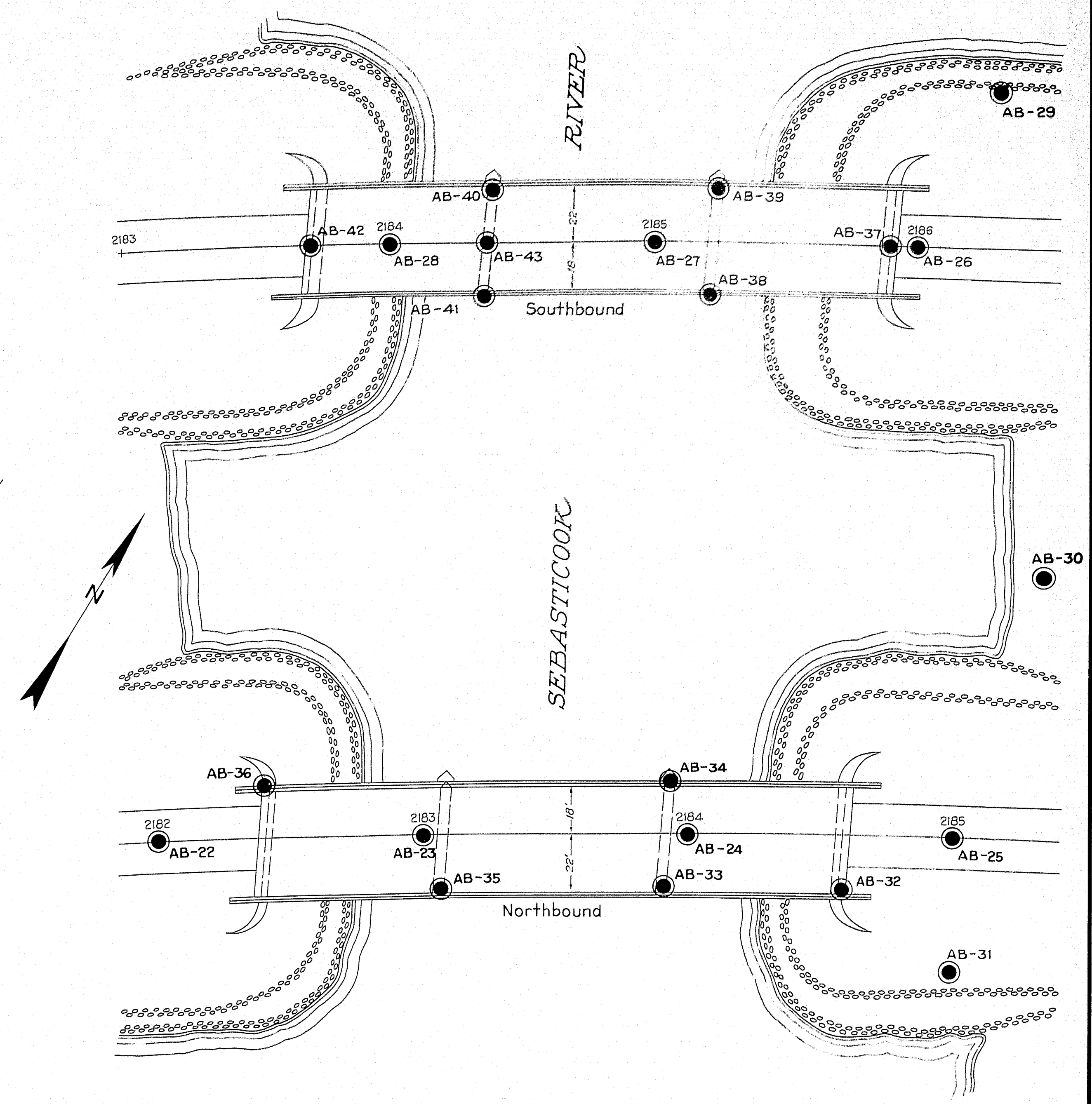
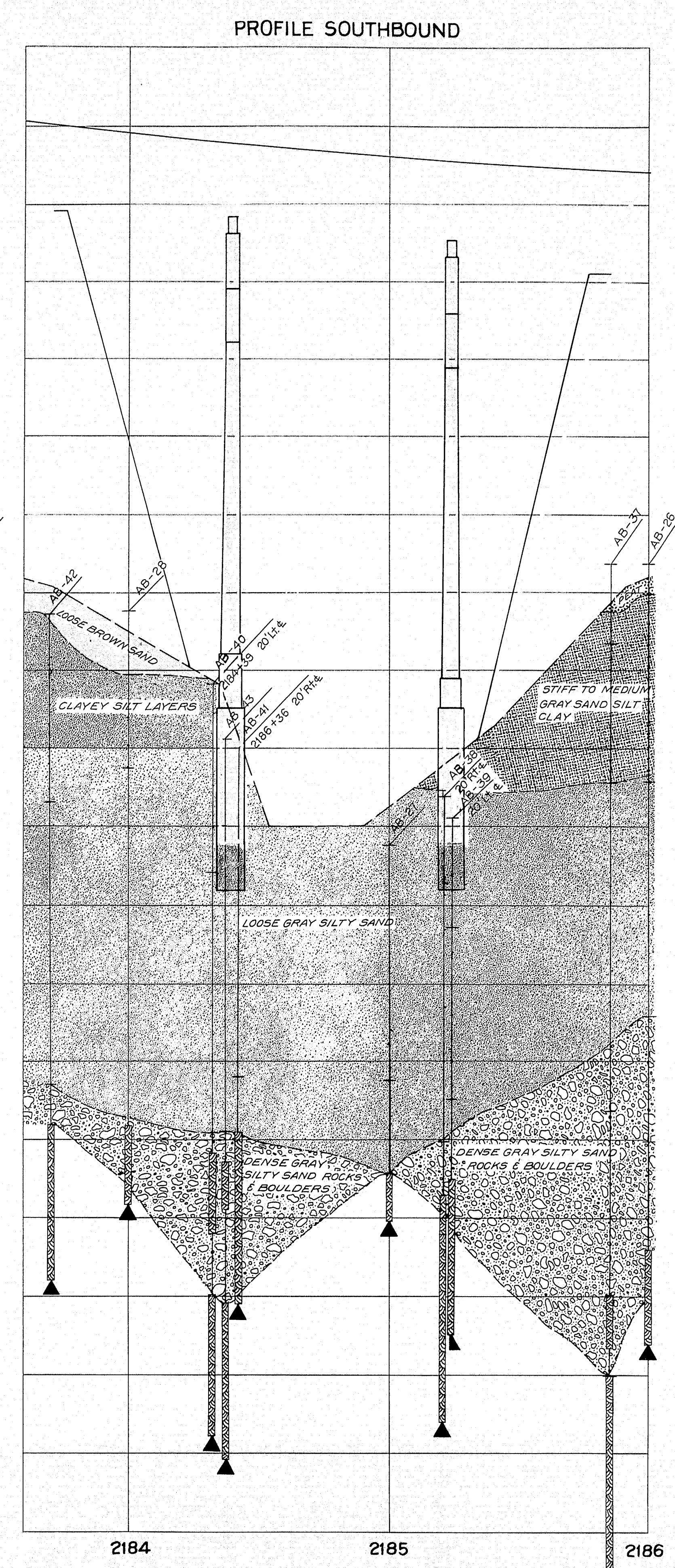
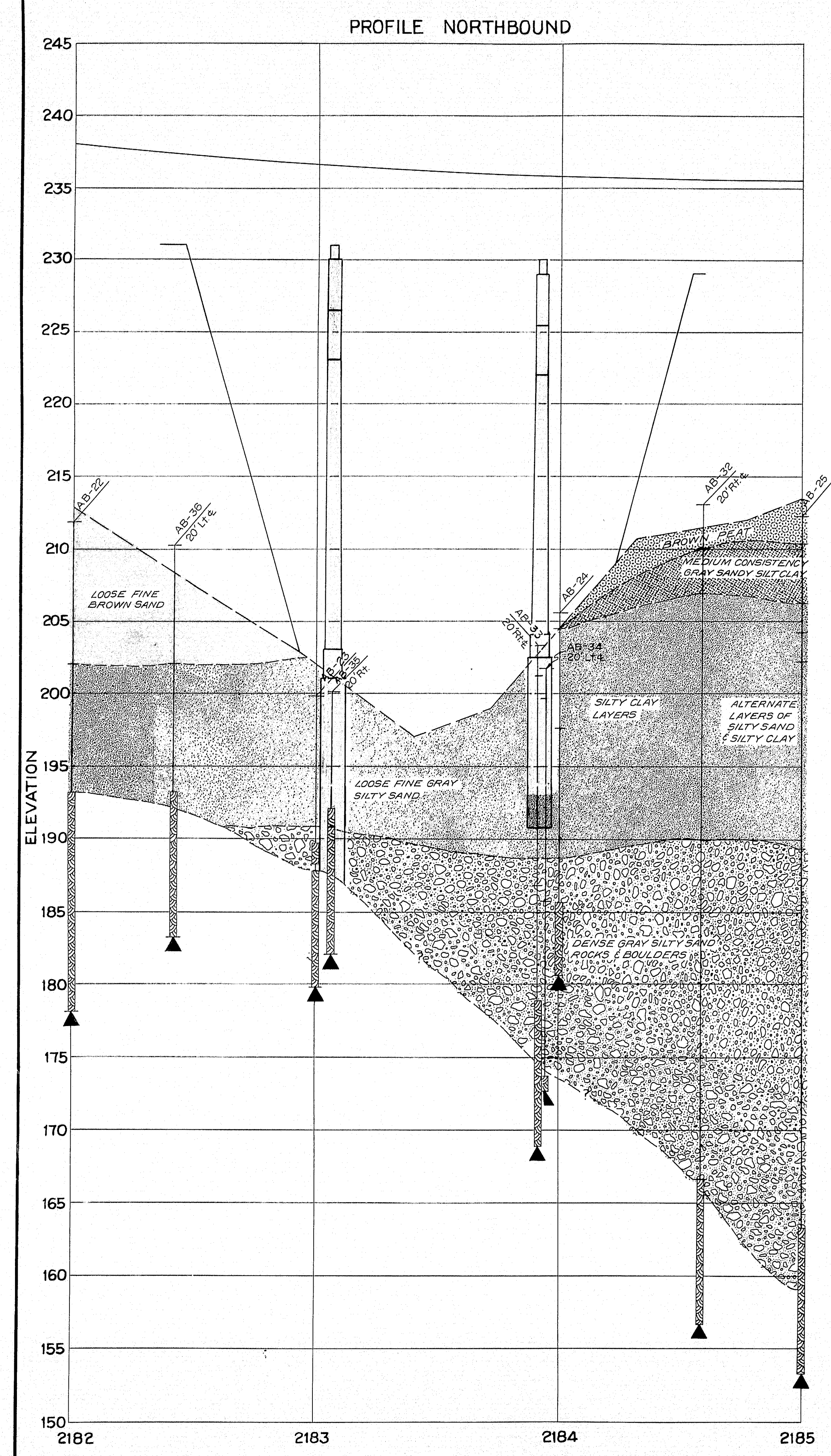


DESIGN - T.H.R.
 TRACE - P. Ault
 CHECK -
 BRIDGE NO. 1
 SURVEY - BLAKE
 PLOT - P. Ault
 STATE HIGHWAY COMMISSION
 BRIDGE DIVISION
SEBASTICOOK RIVER BRIDGE
 IN THE TOWN OF
PITTSFIELD
SOMERSET COUNTY
 SURVEY
 SHEET 2 OF 18 AUGUSTA, MAINE JULY 1962

M-1981



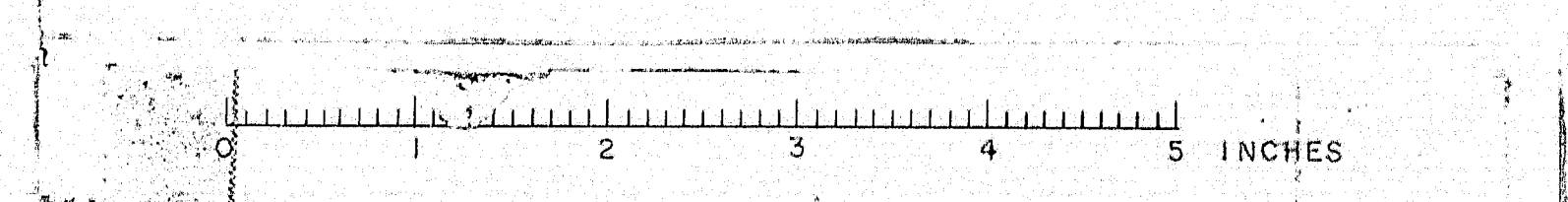
B. P. R. REG. NO.	STATE	PROJECT NUMBER	SHEET NO.	TOTAL SHEETS
1	MAINE	I-95-7(35)	47	225



SCALE 1"=30'

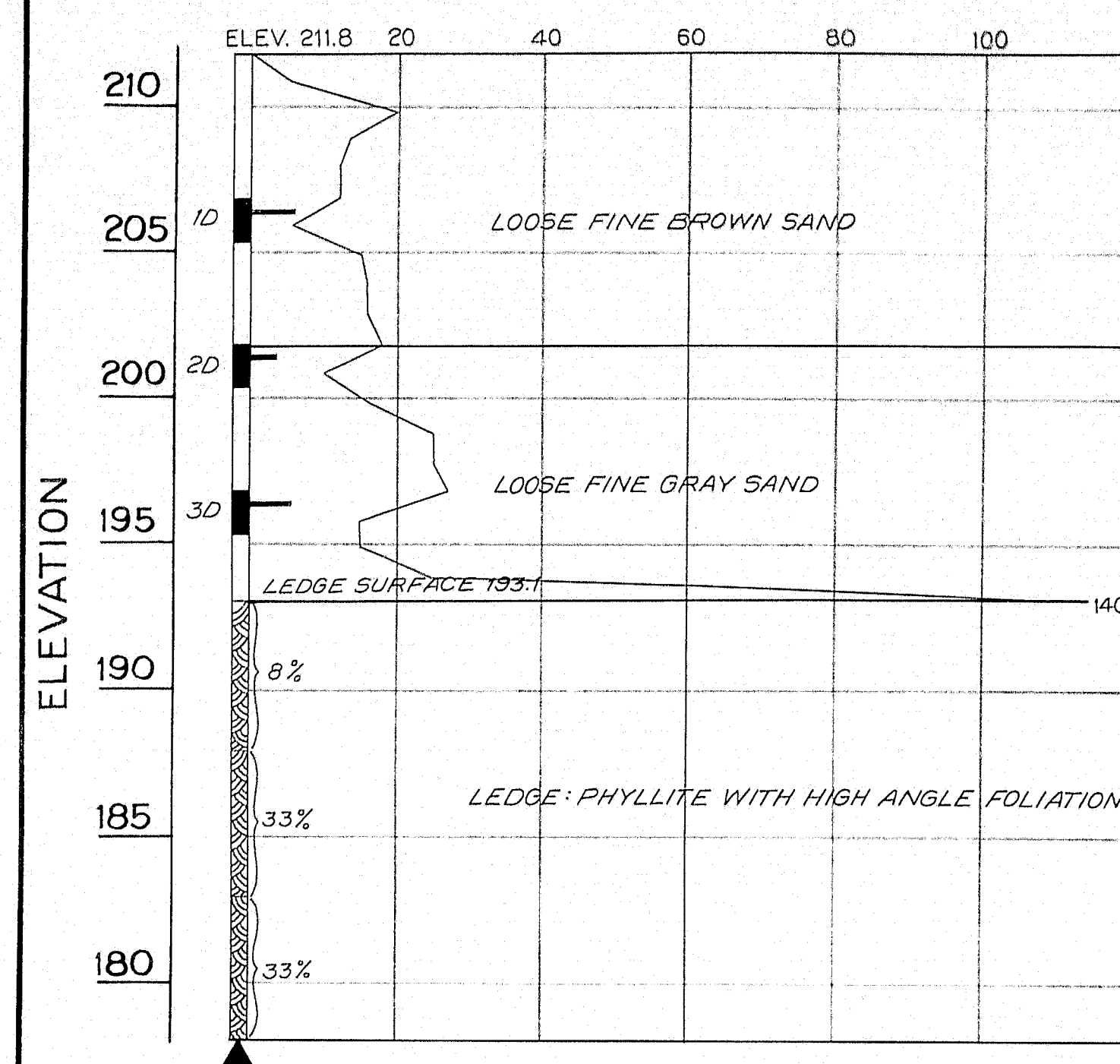
DESIGN-- TRACE-- CHECK--	BRIDGE I.O. SURVEY-- PLOT--
STATE HIGHWAY COMMISSION BRIDGE DIVISION	
SEBASTICOOK RIVER BRIDGE	
IN THE TOWN OF	
PITTSFIELD	
SOMERSET COUNTY	
FOUNDATION SURVEY	
SHEET 3 OF 18 AUGUSTA, MAINE	

MAY 1982



BORING AB-22 STATION 2182+0 & N.B.L.

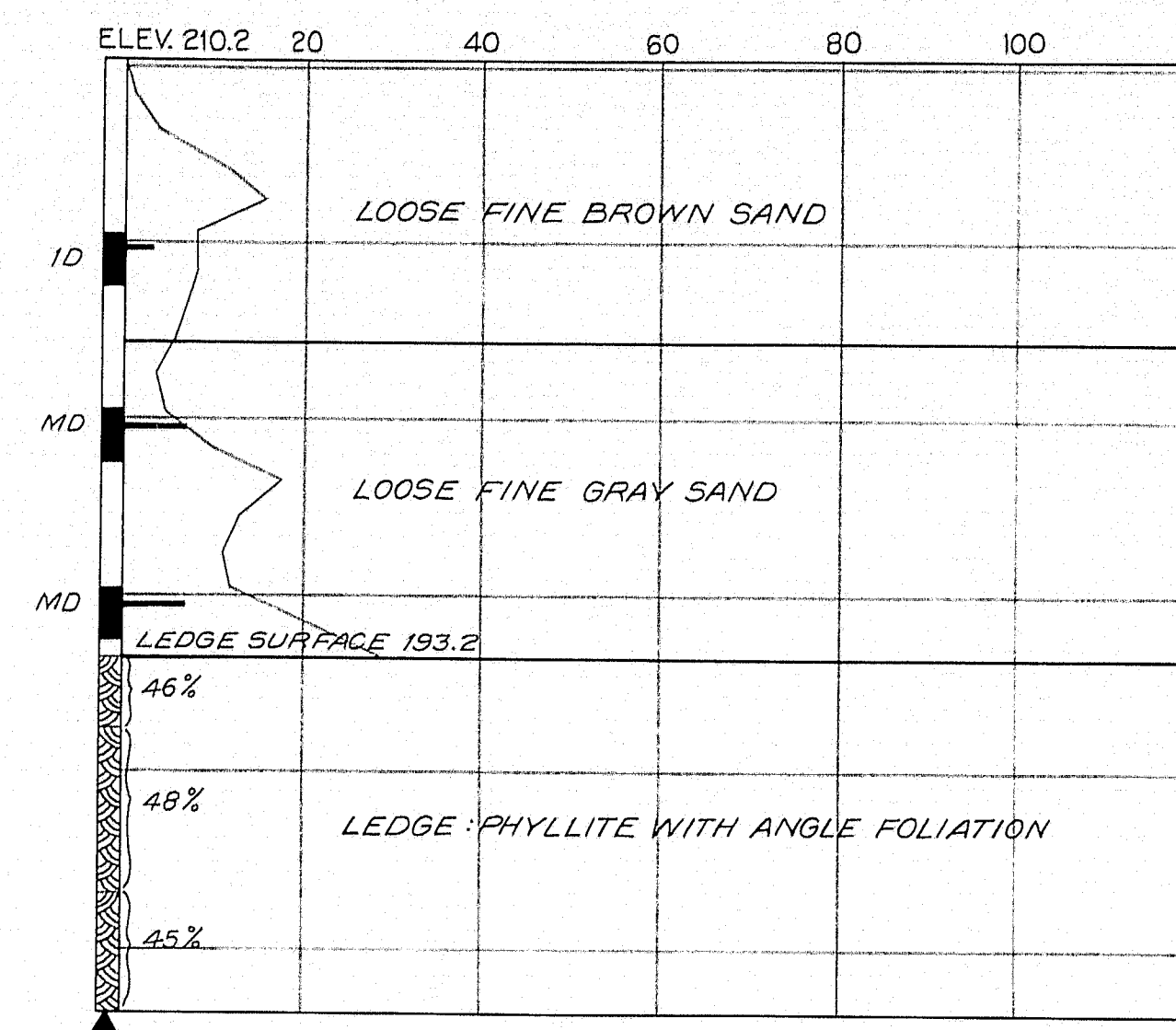
CASING SIZE 2 1/2"



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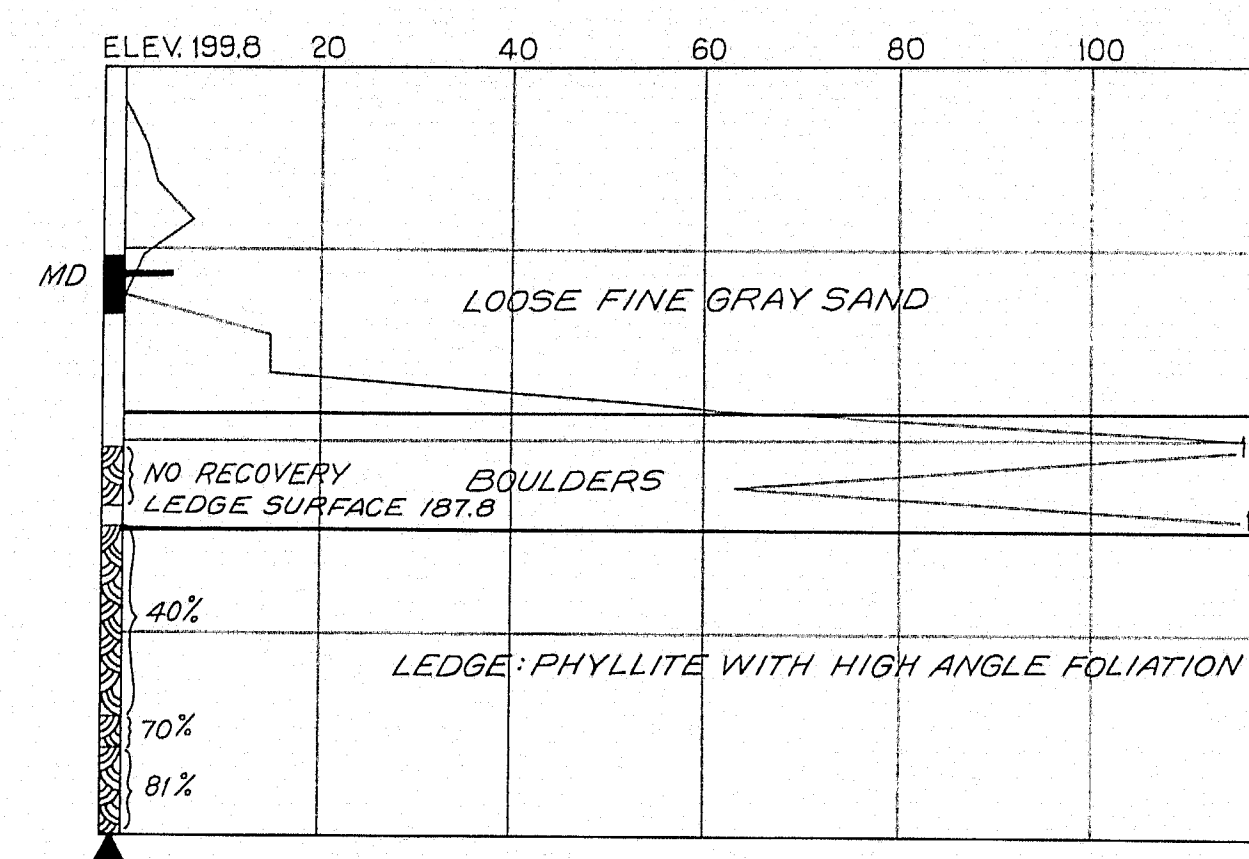
CASING SIZE 2 1/2"

ABUTMENT NO. 1



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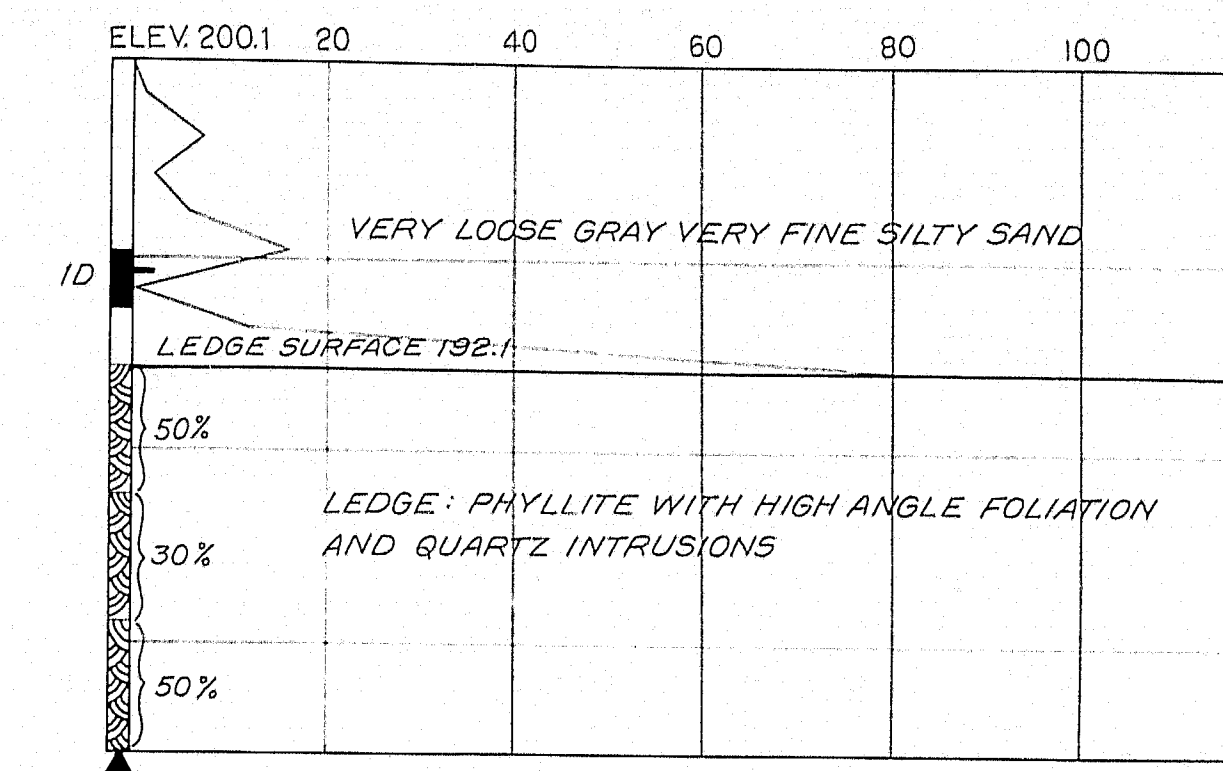
CASING SIZE 2 1/2"



BORING AB-35 STATION 2183+06 20' Rt. N.B.L.

CASING SIZE 2 1/2"

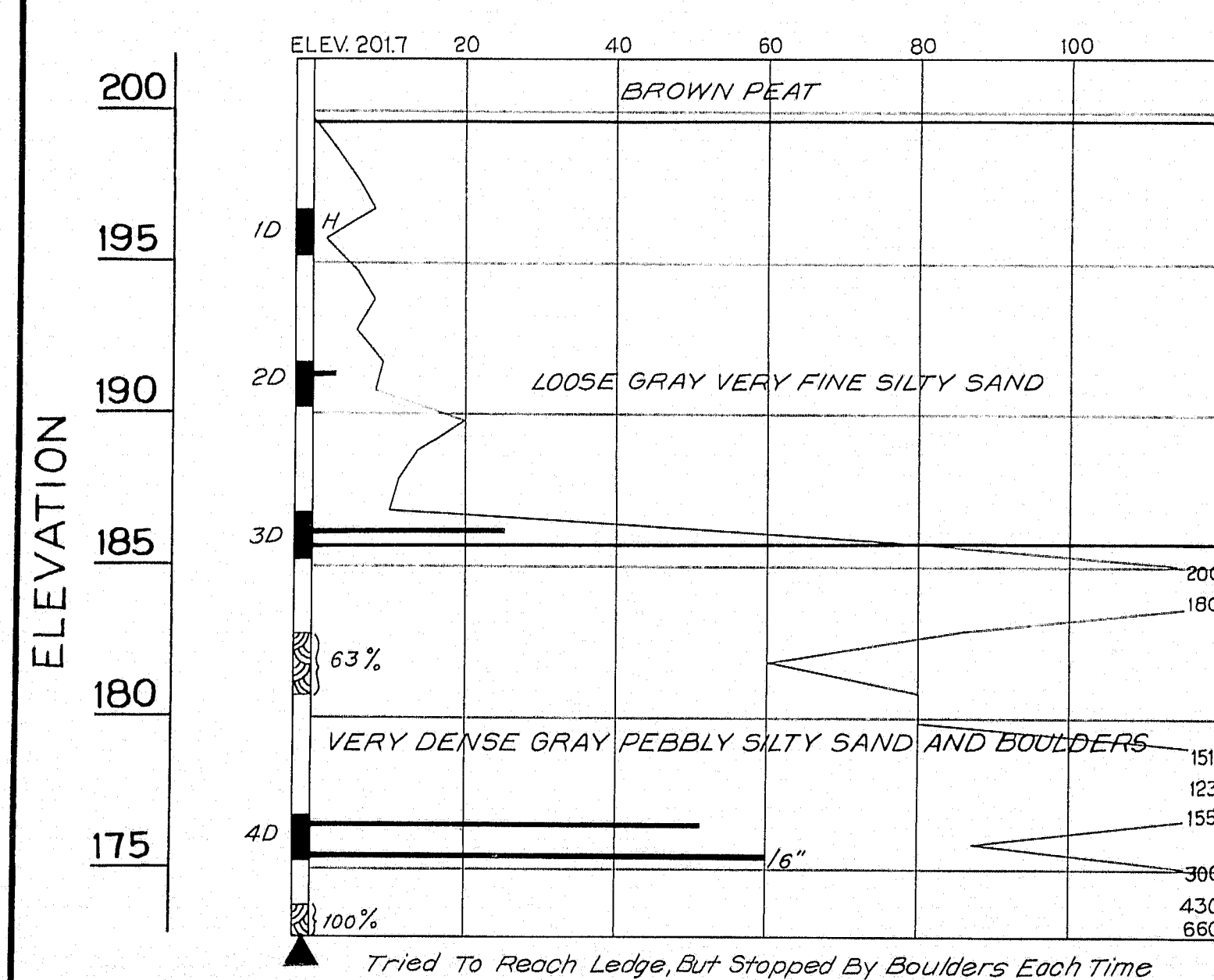
PIER NO. 1



BORING AB-34 STATION 2183+94 20' Lt. & N.B.

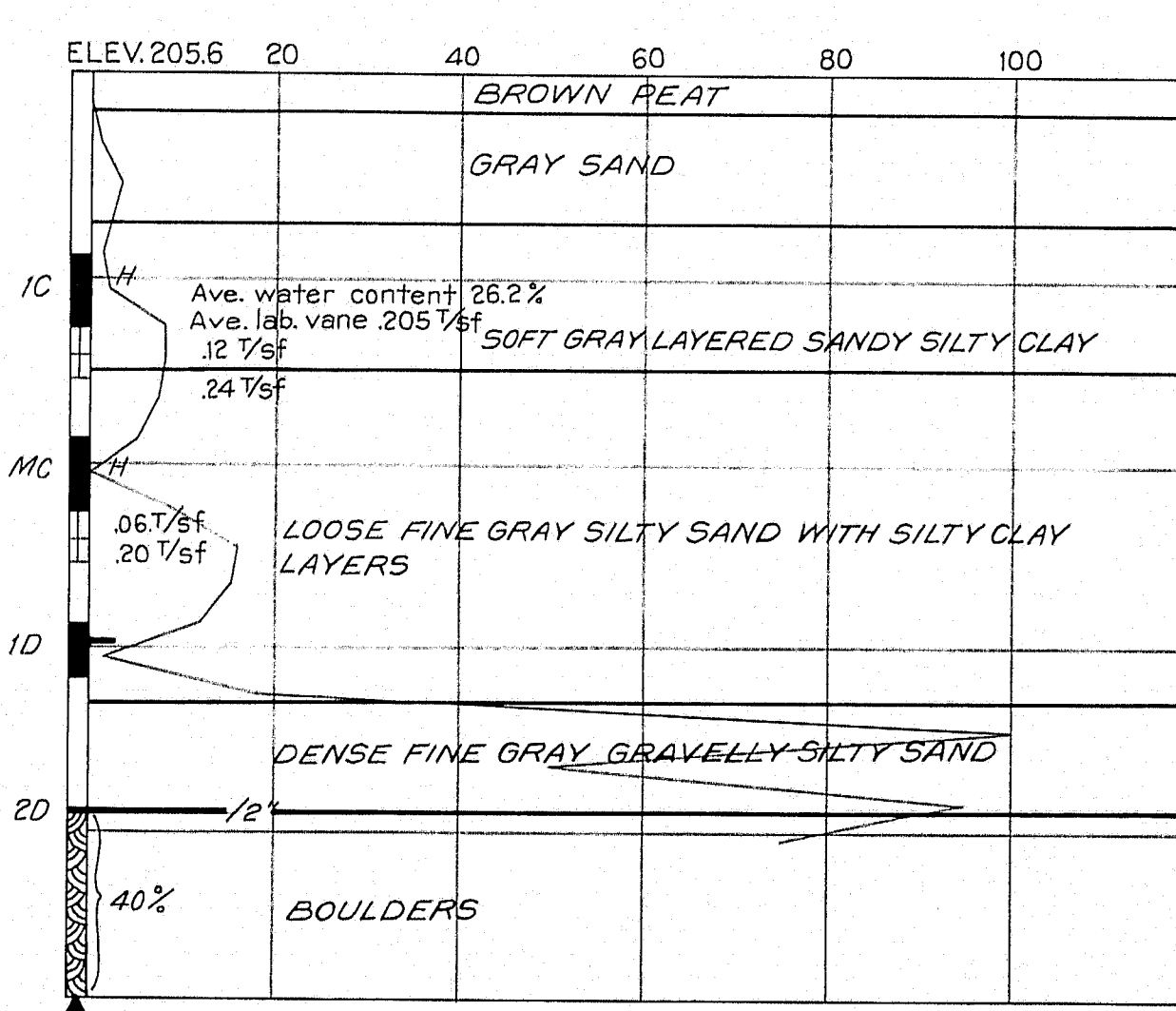
CASING SIZE 2 1/2"

PIER NO. 2



BORING AB-24 STATION 2184+0 & N.B.

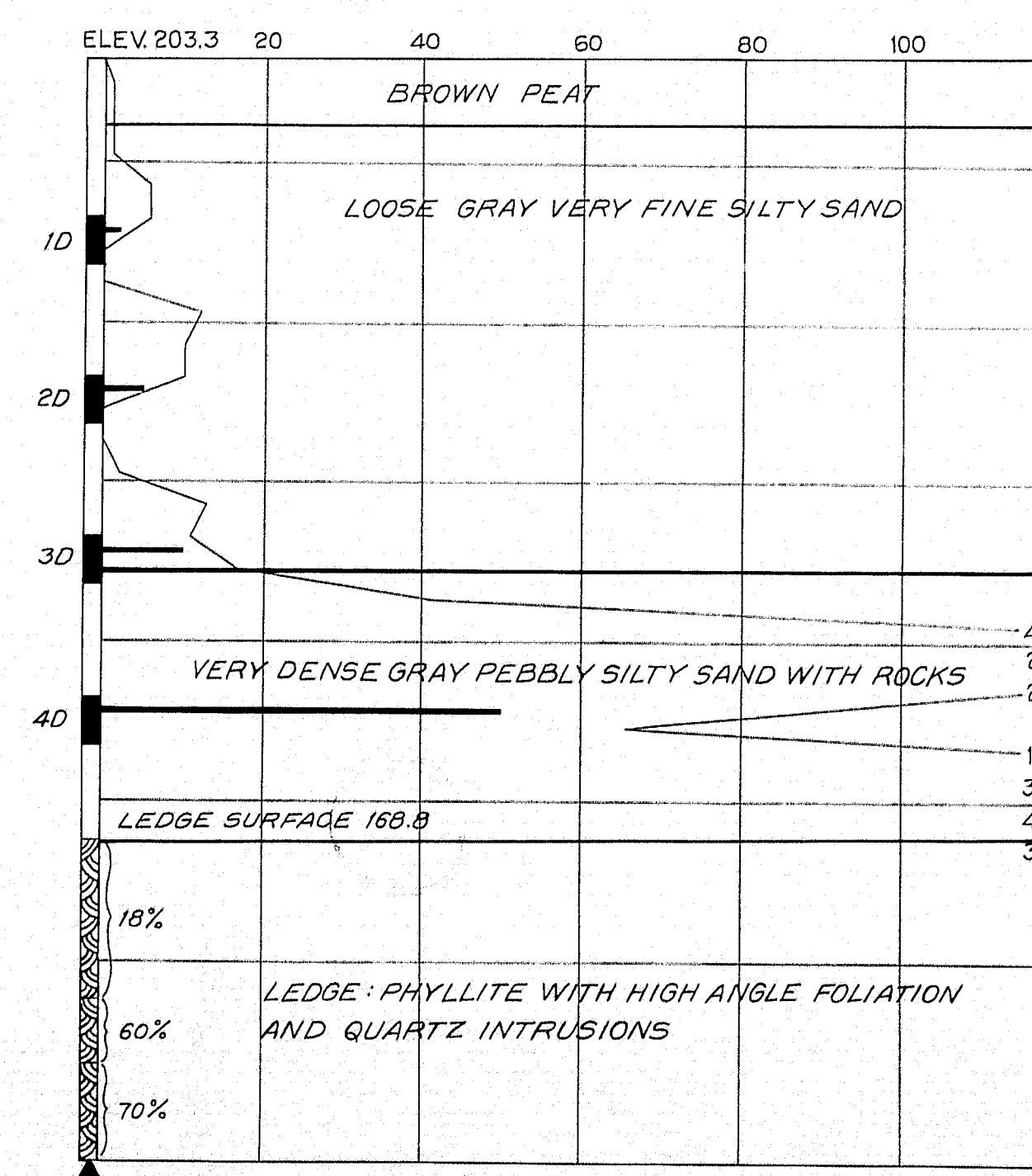
CASING SIZE 2 1/2"



BORING AB-33 STATION 2183+91 20' Rt. & N.B.

CASING SIZE 2 1/2"

PIER NO. 2



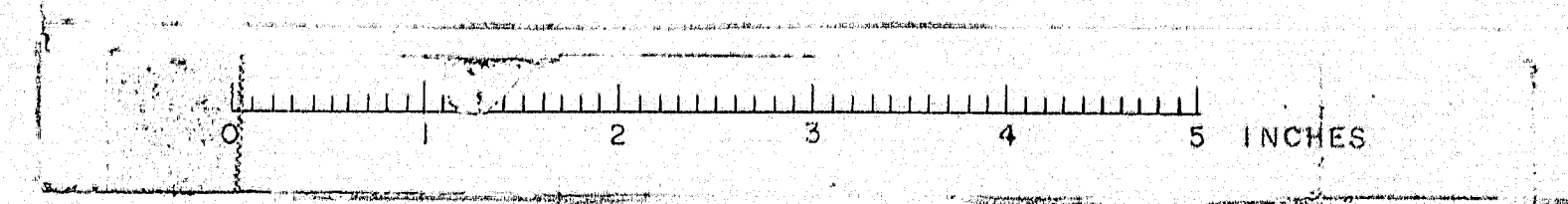
BORING NOTES

NUMBER OF BLOWS REQUIRED TO DRIVE EXTRA HEAVY CASING ONE FOOT WITH 400 FT. LBS. OF ENERGY PER BLOW
LOCATION OF SAMPLE OR SAMPLE ATTEMPT
S & H SAMPLER # 1290'S
2" O.D. 16 GA. SEAMLESS TUBING
UNSUCCESSFUL SAMPLE ATTEMPT AND TYPE OF SAMPLER
NUMBER OF BLOWS REQUIRED TO DRIVE SPOON OR TUBING ONE FOOT WITH 350 FT. LBS. OF ENERGY PER BLOW
SAMPLING SPOON OR SEAMLESS TUBING DRIVEN BY STATIC WEIGHT OF DRILL RODS AND HAMMER
FIELD VANE TEST
BOTTOM OF BORING (MAY NOT BE BOTTOM OF SOIL STRATA)
LOCATIONS CORED BY DIAMOND BIT AND PER CENT RECOVERY OF ROCK

DESIGN-- TRACE-- CHECK--	BRIDGE NO. SURVEY-- PLOT--
STATE HIGHWAY COMMISSION BRIDGE DIVISION	
SEBASTICOOK RIVER BRIDGE	
IN THE TOWN OF PITTSFIELD SOMERSET COUNTY	
BORING DETAILS	

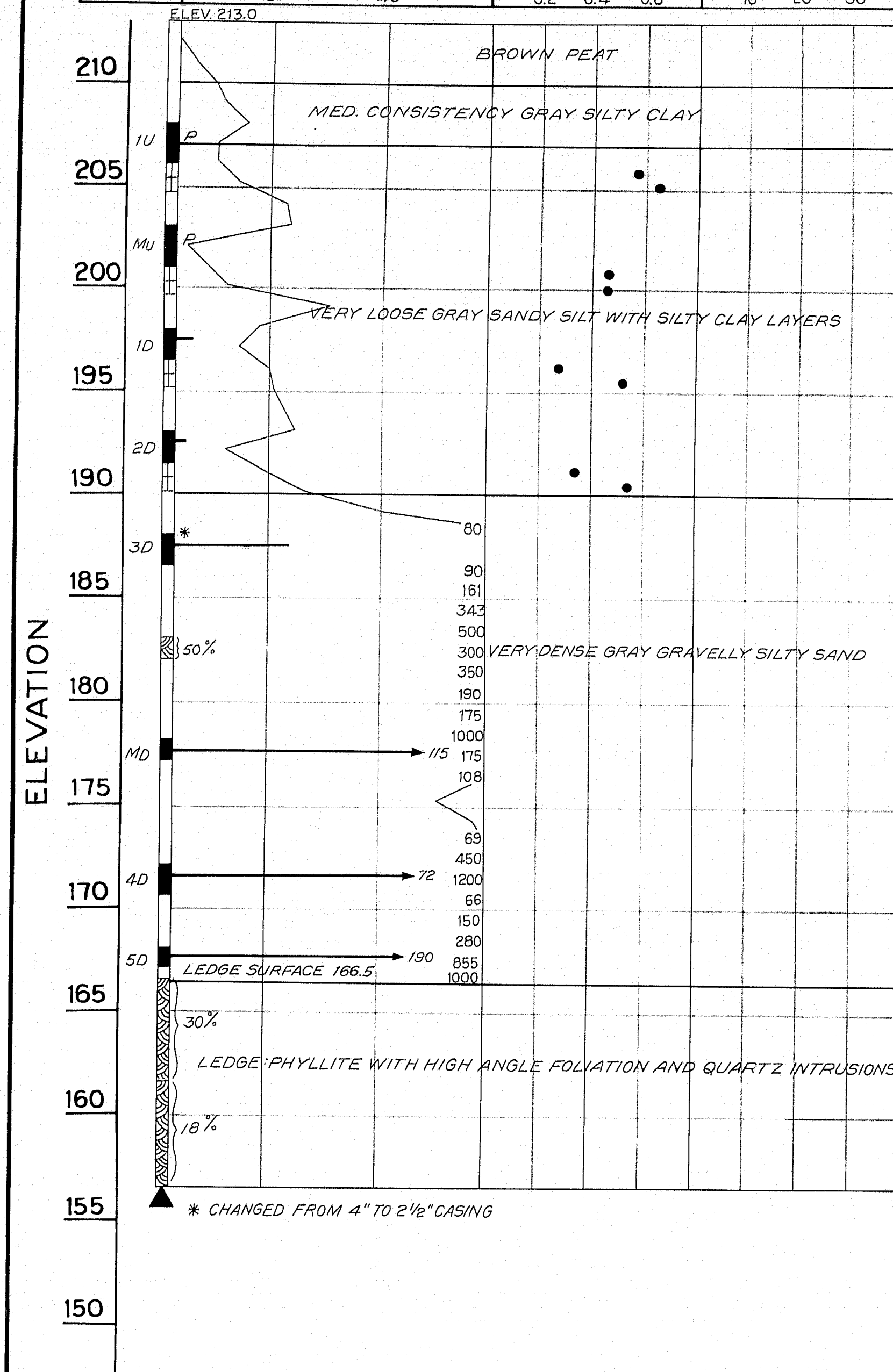
SHEET 4 OF 18 AUGUSTA, MAINE

M-1983



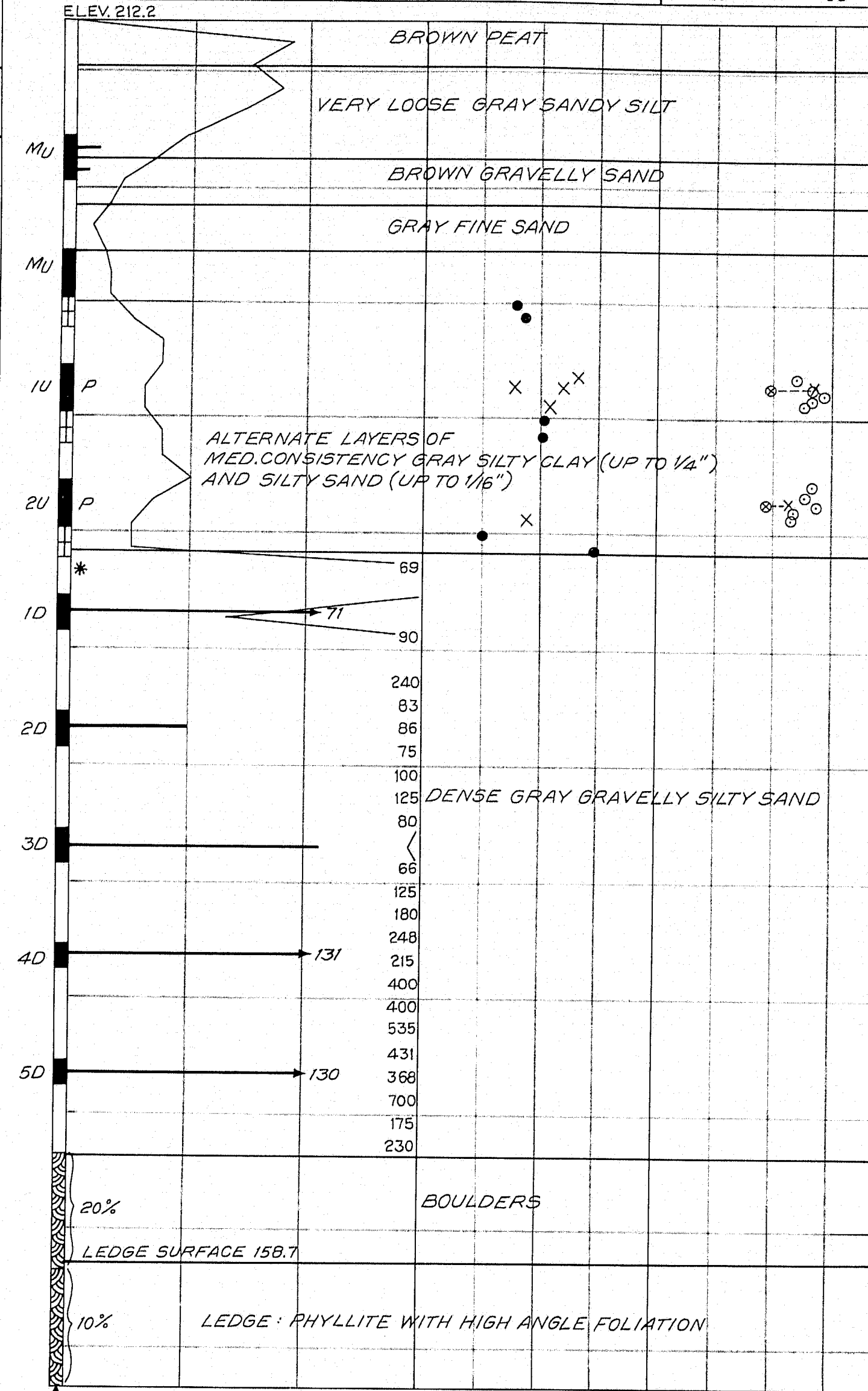
ABUTMENT NO. 2

BORING AB-32 STATION 2184+59 20' Rt. & N.B.				
CASING SIZE	DRIVING RESISTANCE	VANE SHEAR STRENGTH	WATER CONTENT	
4" & 2 1/2"	Blows/Ft.	Tons/Sq. Ft.	Percent	
	20 40	0.2 0.4 0.6	10 20 30	



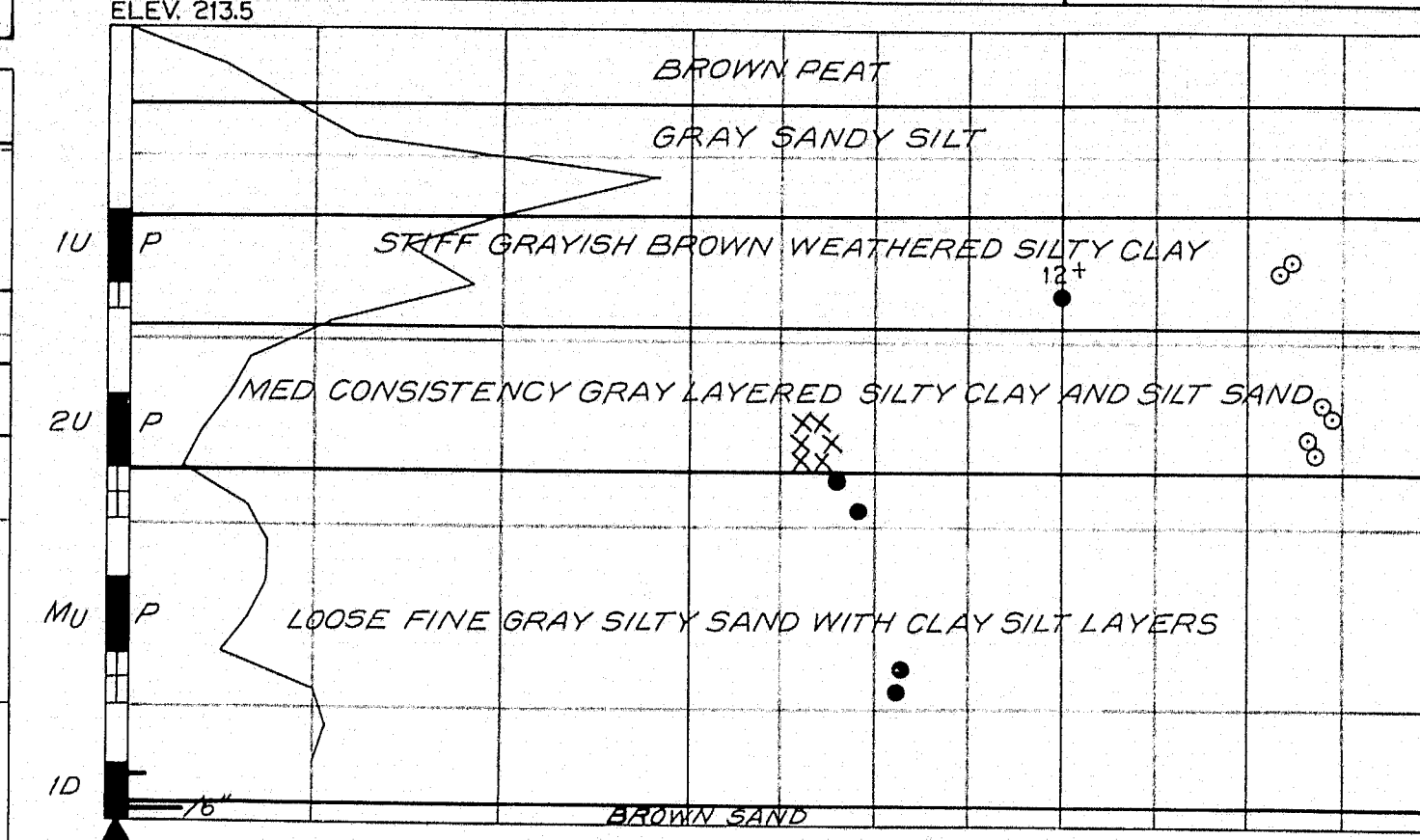
BORING AB-25 STATION 2185+0 & N.B.

DRIVING RESISTANCE	VANE SHEAR STRENGTH	WATER CONTENT
Blows/Ft.	Tons/Sq. Ft.	Percent
20 40	0.2 0.4 0.6	10 20 30



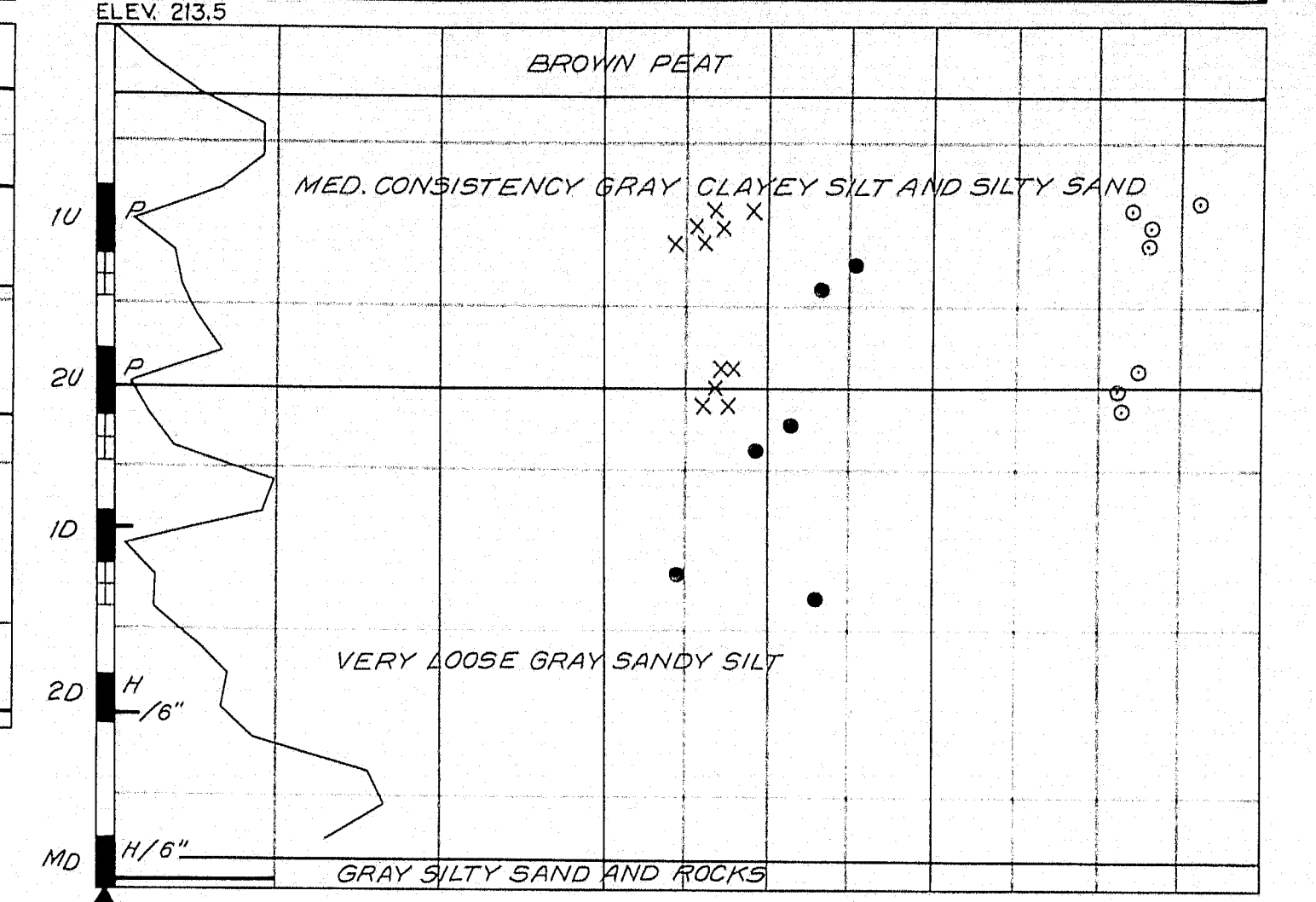
BORING AB-30 STATION 2185+30 100 Lt. & N.B.

DRIVING RESISTANCE	VANE SHEAR STRENGTH	WATER CONTENT
Blows/Ft.	Tons/Sq. Ft.	Percent
20 40	0.2 0.4 0.6	10 20 30



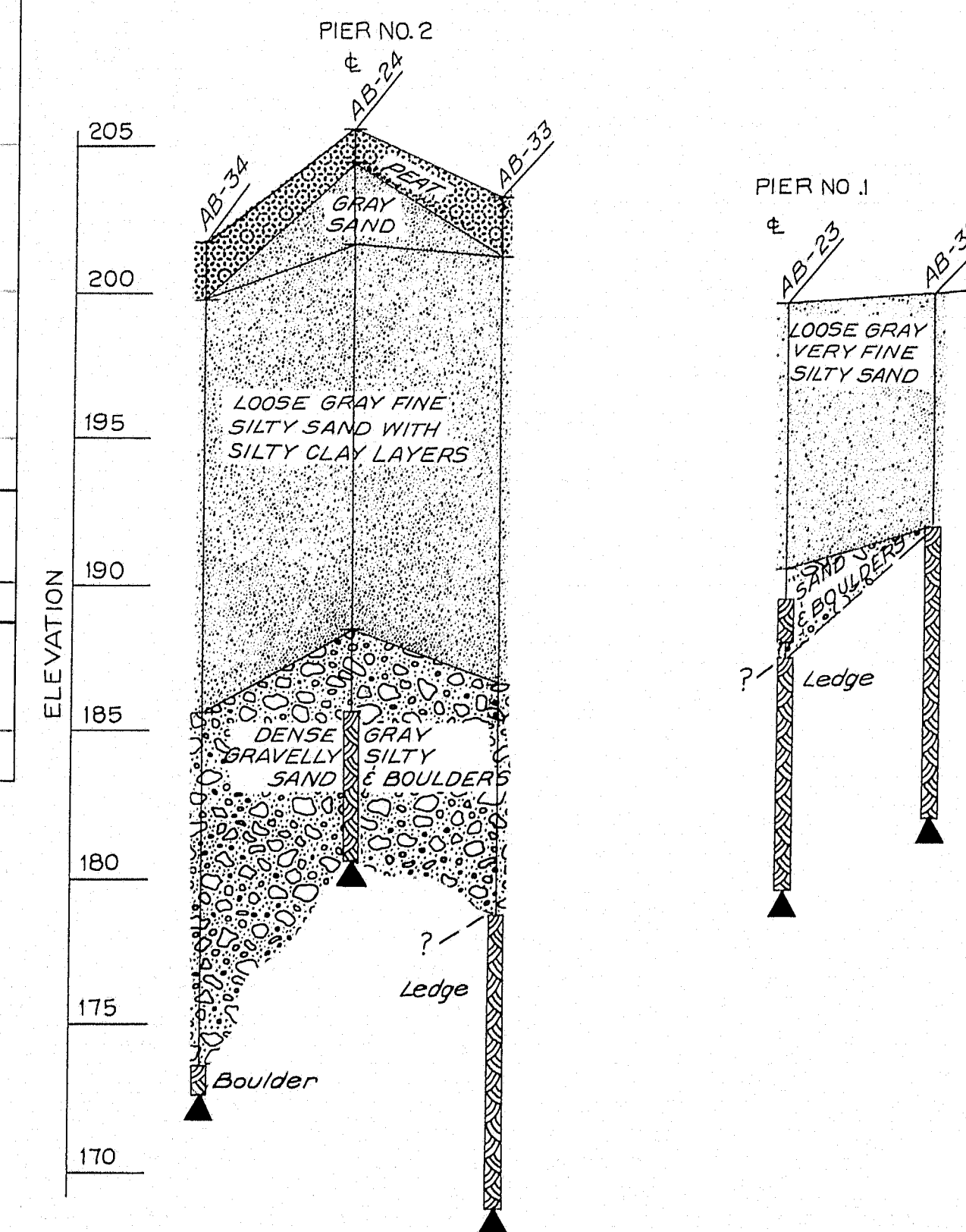
BORING AB-31 STATION 2185+0 50' Rt. & N.B.

DRIVING RESISTANCE	VANE SHEAR STRENGTH	WATER CONTENT
Blows/Ft.	Tons/Sq. Ft.	Percent
20 40	0.2 0.4 0.6	10 20 30



TRANSVERSE SECTIONS

(NORTHBOUND)

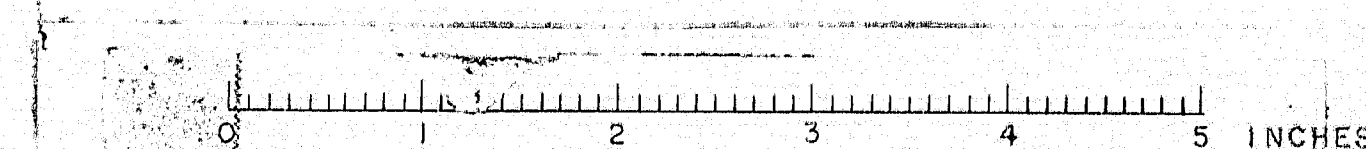


BORING NOTES

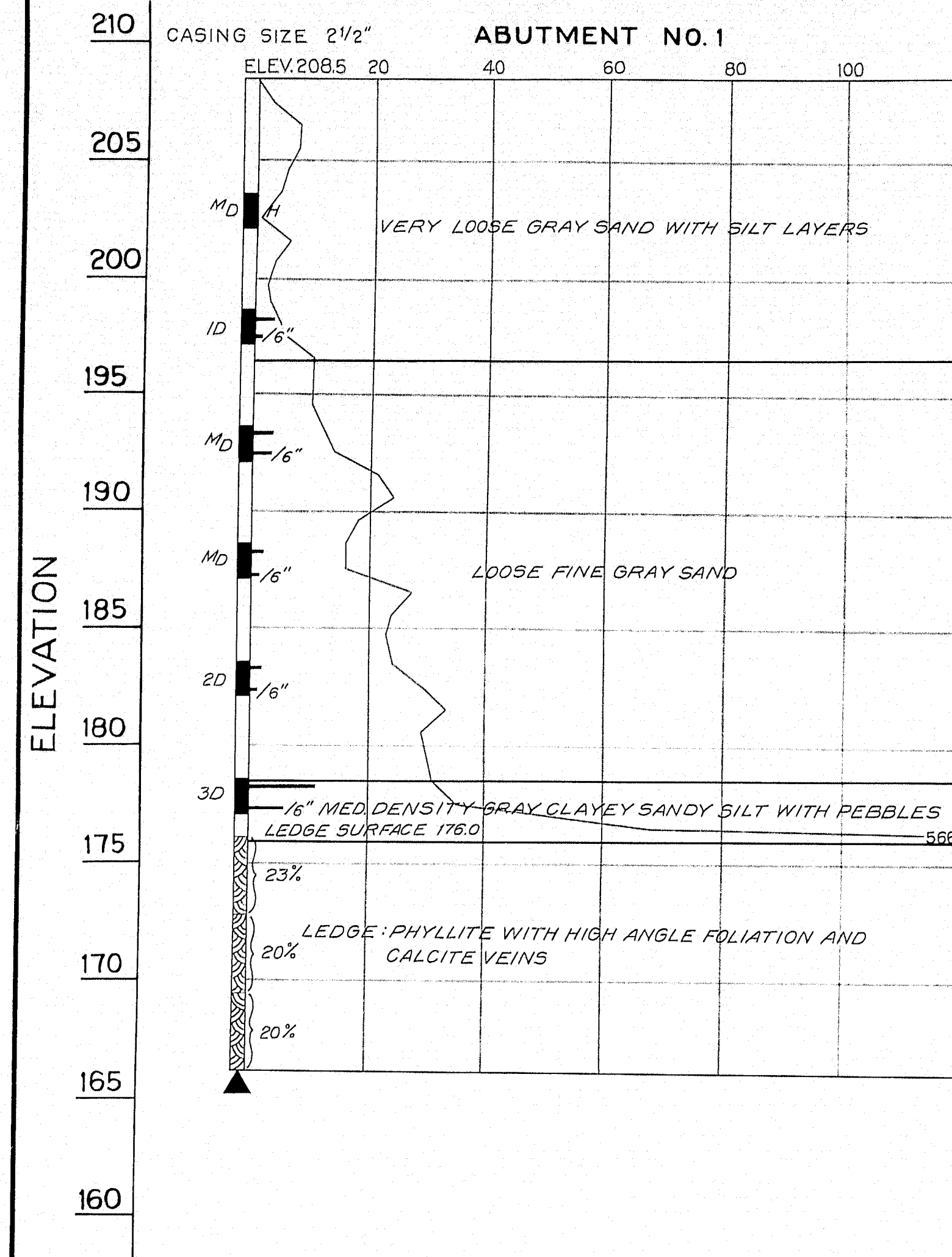
- NUMBER OF BLOWS REQUIRED TO DRIVE EXTRA HEAVY CASING ONE FOOT WITH 400 FT. LBS. OF ENERGY PER BLOW
- LOCATION OF SAMPLE OR SAMPLE ATTEMPT
- S & H SAMPLER #1290'S
- 3 1/2" O.D. 16 GA. SEAMLESS TUBING
- UNSUCCESSFUL SAMPLE ATTEMPT AND TYPE OF SAMPLER
- NUMBER OF BLOWS REQUIRED TO DRIVE SPOON OR TUBING ONE FOOT WITH 350 FT. LBS. OF ENERGY PER BLOW
- SAMPLING SPOON OR SEAMLESS TUBING DRIVING BY STATIC WEIGHT OF DRILL RODS AND HAMMER
- PISTON SAMPLE
- FIELD VANE TEST
- BOTTOM OF BORING (MAY NOT BE BOTTOM OF SOIL STRATA)
- LOCATIONS CORED BY DIAMOND BIT AND PER CENT RECOVERY OF ROCK
- SHEAR NOTES
- FIELD VANE SHEAR STRENGTHS
- LABORATORY VANE SHEAR STRENGTHS
- WATER CONTENT NOTES
- NATURAL WATER CONTENTS, GIVEN AS PER CENT OF DRY WEIGHT
- X PLASTIC AND LIQUID LIMITS

DESIGN- TRACE- CHECK-	BRIDGE NO. SURVEY- PLOT-
STATE HIGHWAY COMMISSION BRIDGE DIVISION	
SEBASTICOOK RIVER BRIDGE	
IN THE TOWN OF PITTSFIELD SOMERSET COUNTY	
BORING DETAILS	
SHEET 5 OF 18 AUGUSTA, MAINE	

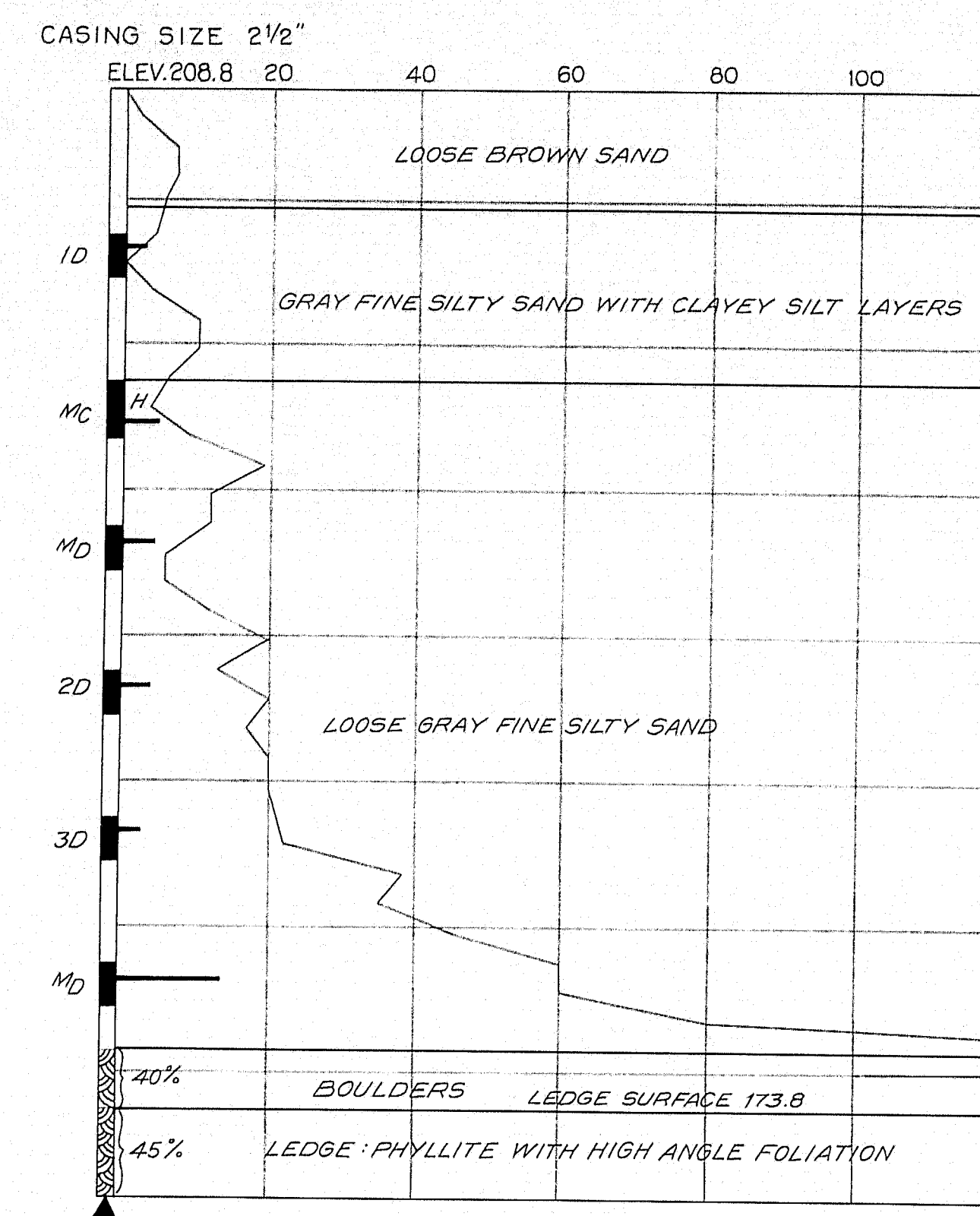
M-1984



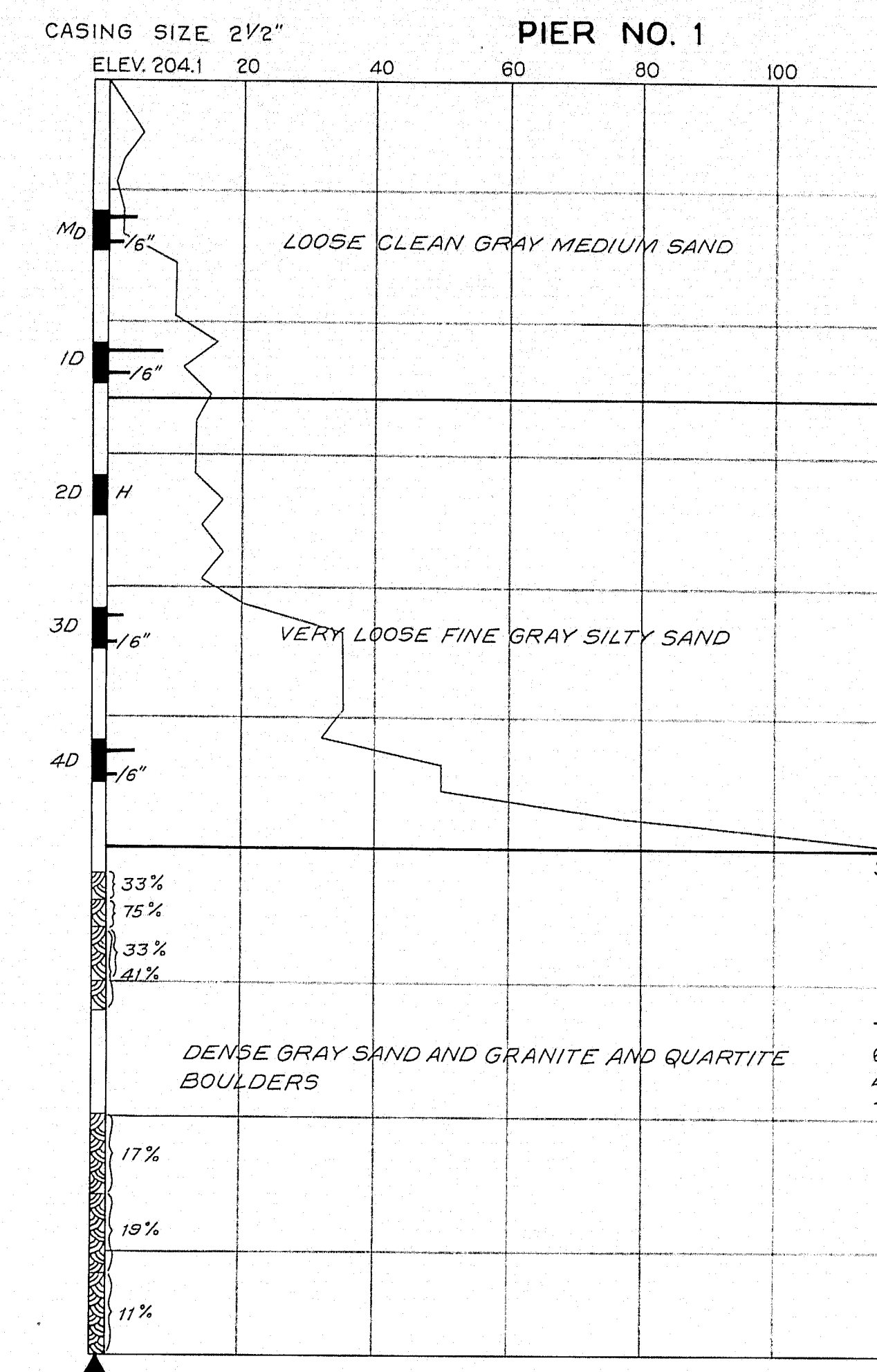
BORING AB-42 STATION 2183+70 & S.B.



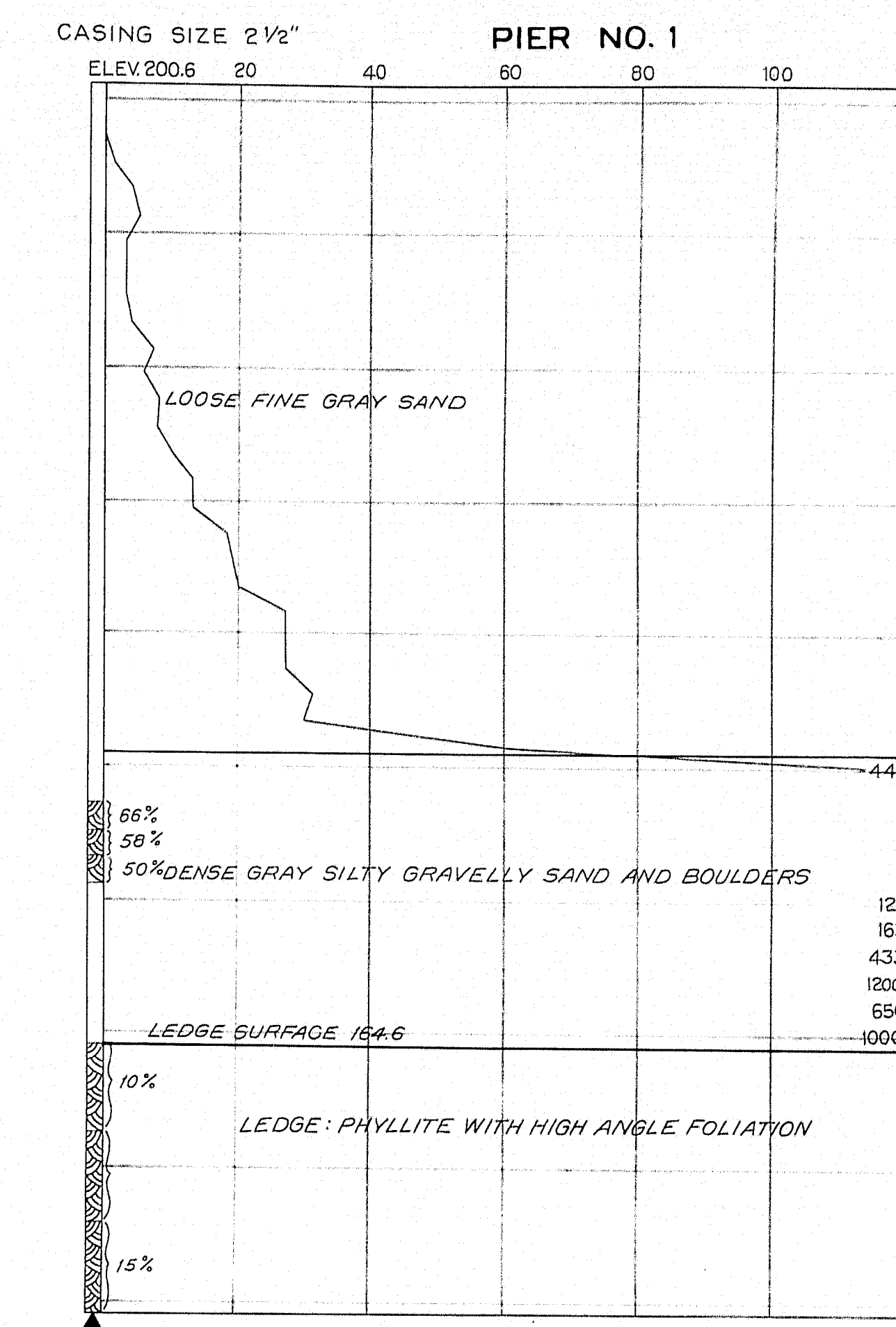
BORING AB-28 STATION 2184+0 & S.B.



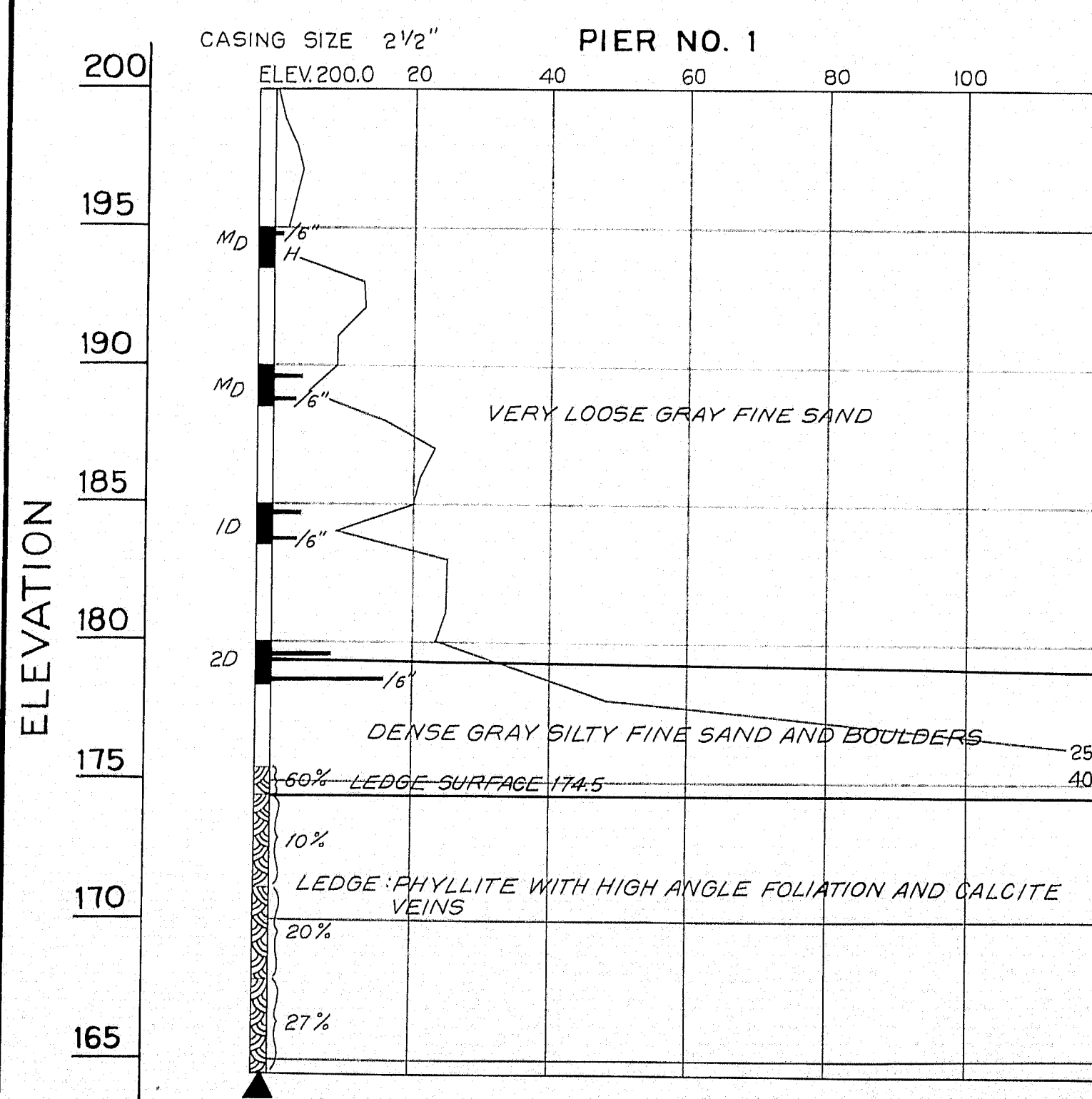
BORING AB-40 STATION 2184+39 20' Lt. S.B.



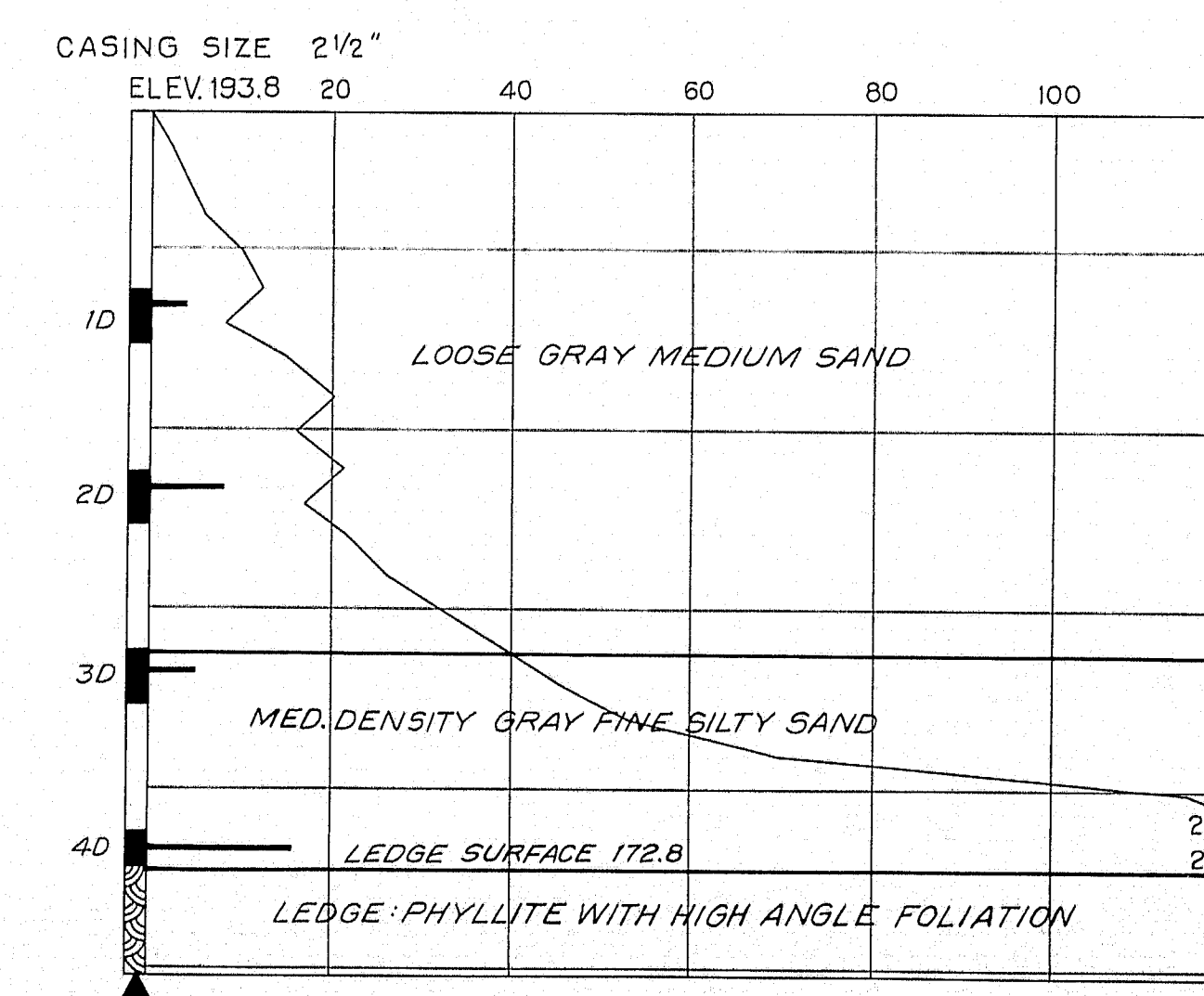
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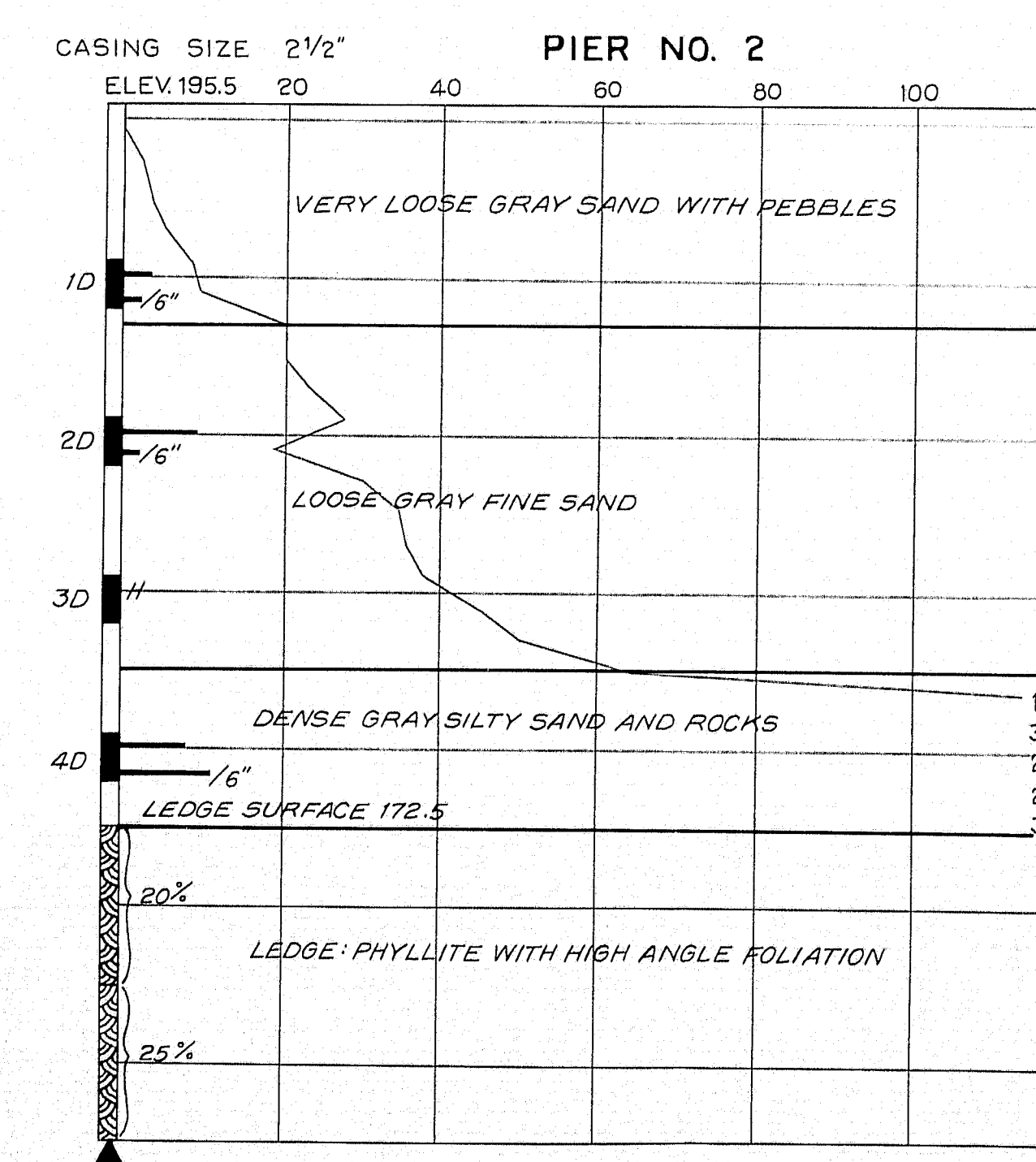
BORING AB-41 STATION 2184+36 20' Rt. S.B.



BORING AB-27 STATION 2185+0 & S.B.



BORING AB-39 STATION 2185+24 20' Lt. S.B.



BORING NOTES

NUMBER OF BLOWS REQUIRED TO DRIVE EXTRA HEAVY CASING ONE FOOT WITH 400 FT. LBS. OF ENERGY PER BLOW.

LOCATION OF SAMPLE OR SAMPLE ATTEMPT

S & H SAMPLE # 1290'S

UNSUCCESSFUL SAMPLE ATTEMPT AND TYPE OF SAMPLER

NUMBER OF BLOWS REQUIRED TO DRIVE SPOON OR TUBING ONE FOOT WITH 350 FT. LBS. OF ENERGY PER BLOW

SAMPLING SPOON OR SEAMLESS TUBING DRIVEN BY STATIC WEIGHT OF DRILL RODS AND HAMMER

BOTTOM OF BORING (MAY NOT BE BOTTOM OF SOILS STRATA)

LOCATIONS CORED BY DIAMOND BIT AND PER CENT RECOVERY OF ROCK

DESIGN--
TRACE--
CHECK--

BRIDGE NO.
SURVEY--
PILOT--

STATE HIGHWAY COMMISSION
BRIDGE DIVISION

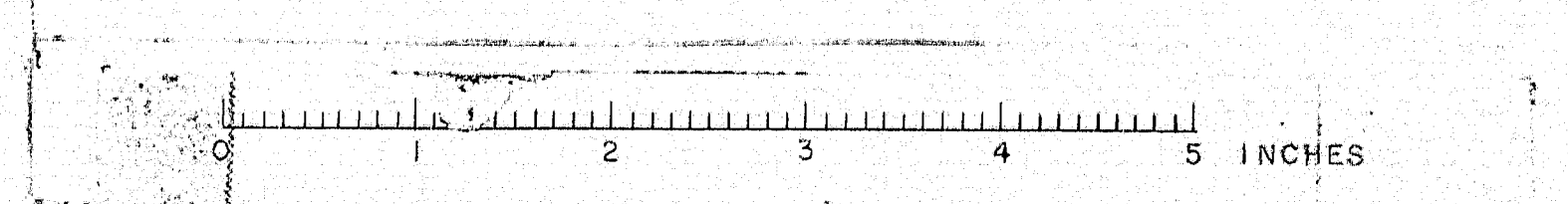
SEBASTICOOK RIVER BRIDGE

IN THE TOWN OF
PITTSFIELD
SOMERSET COUNTY

BORING DETAILS

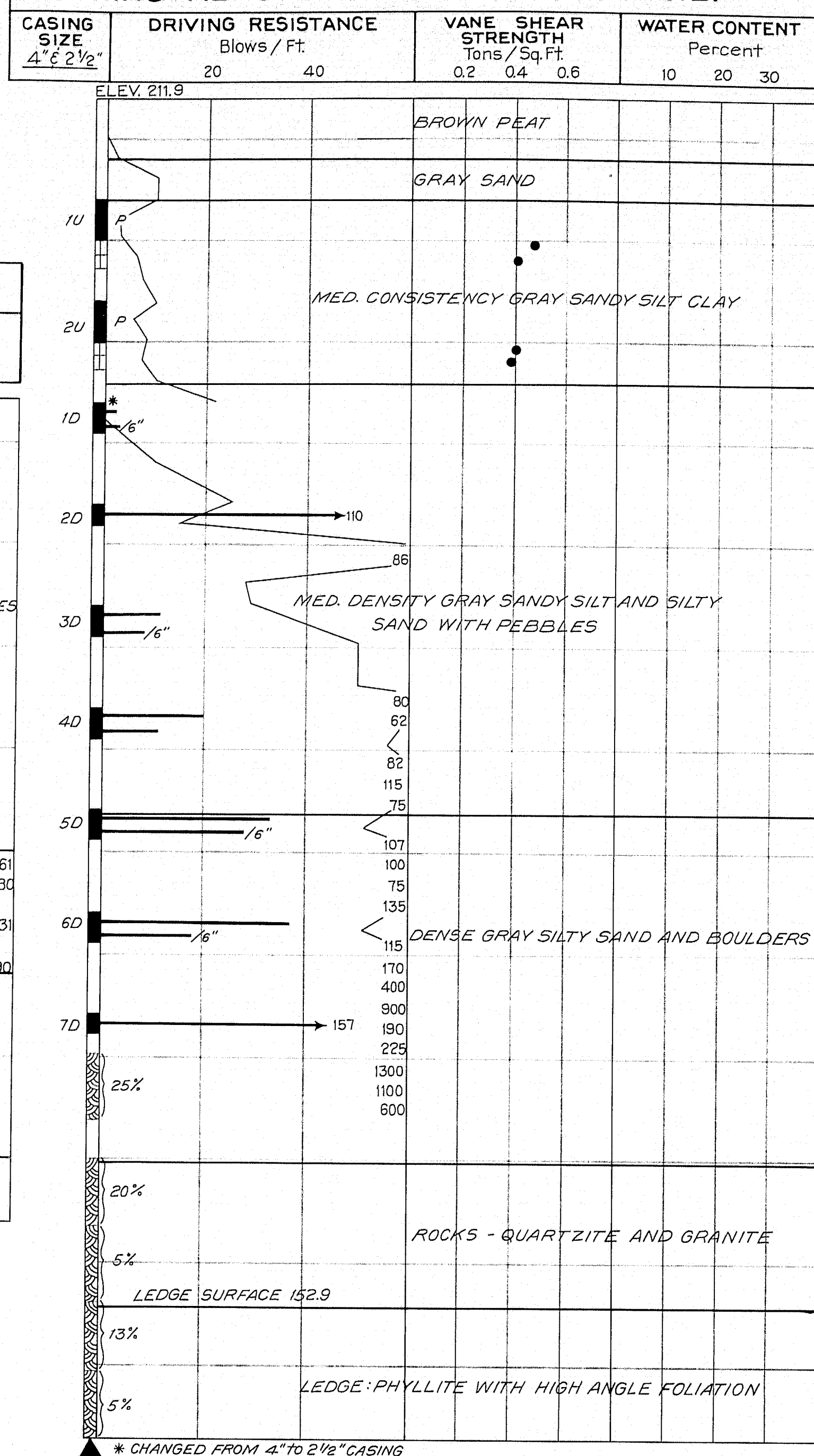
SHEET 6 OF 18 AUGUSTA, MAINE

M-1985

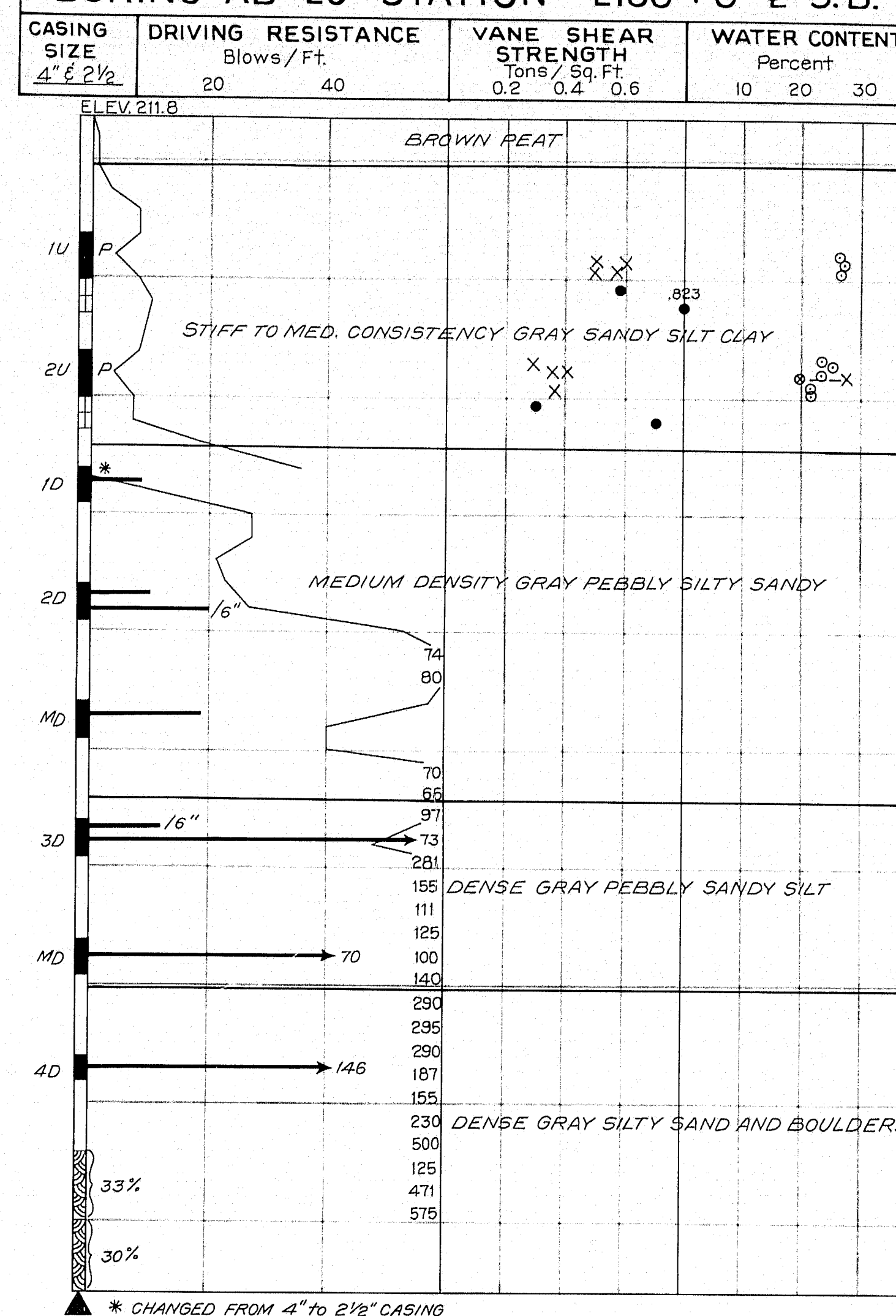


ABUTMENT NO. 2

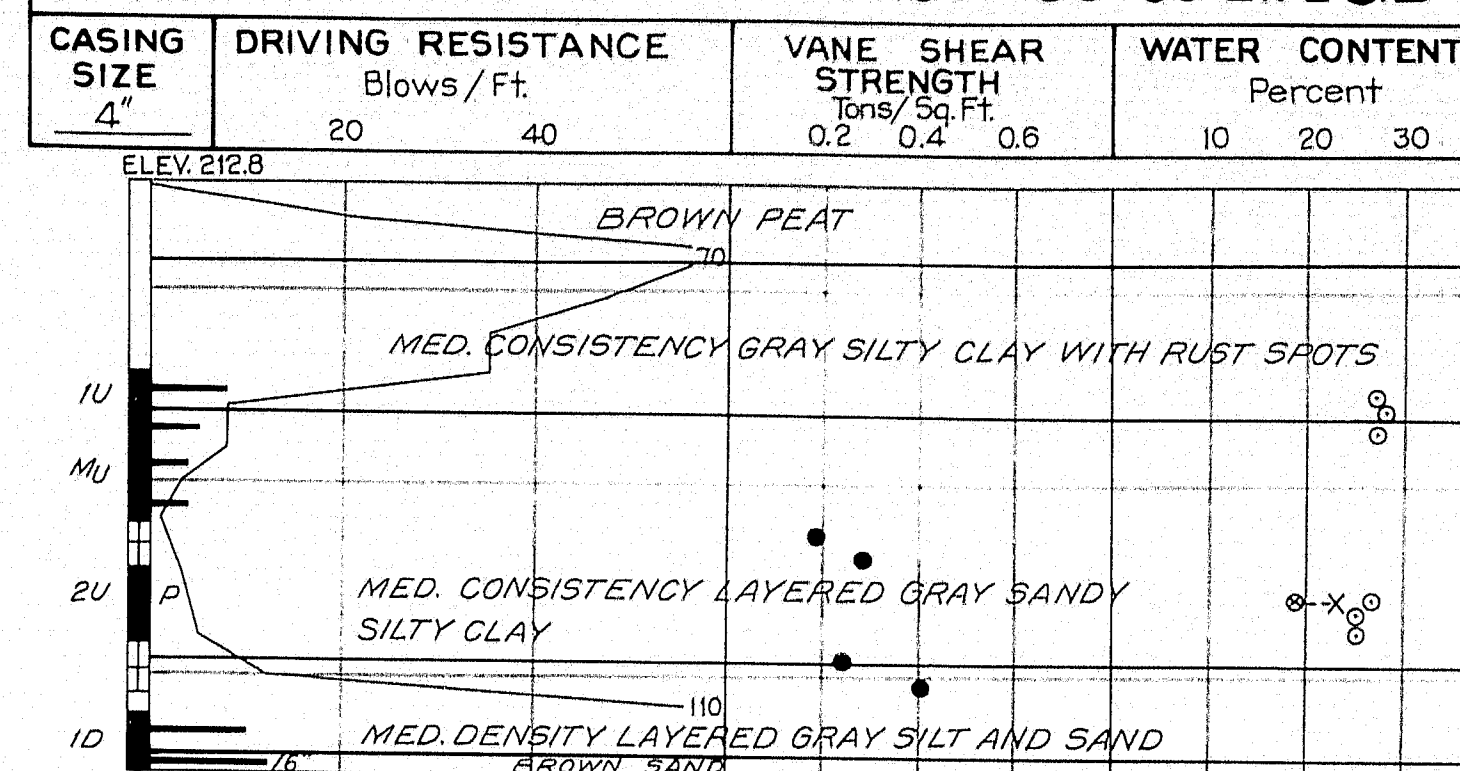
BORING AB-37 STATION 2185 90 S.B.



BORING AB-26 STATION 2186 +0 & S.B.

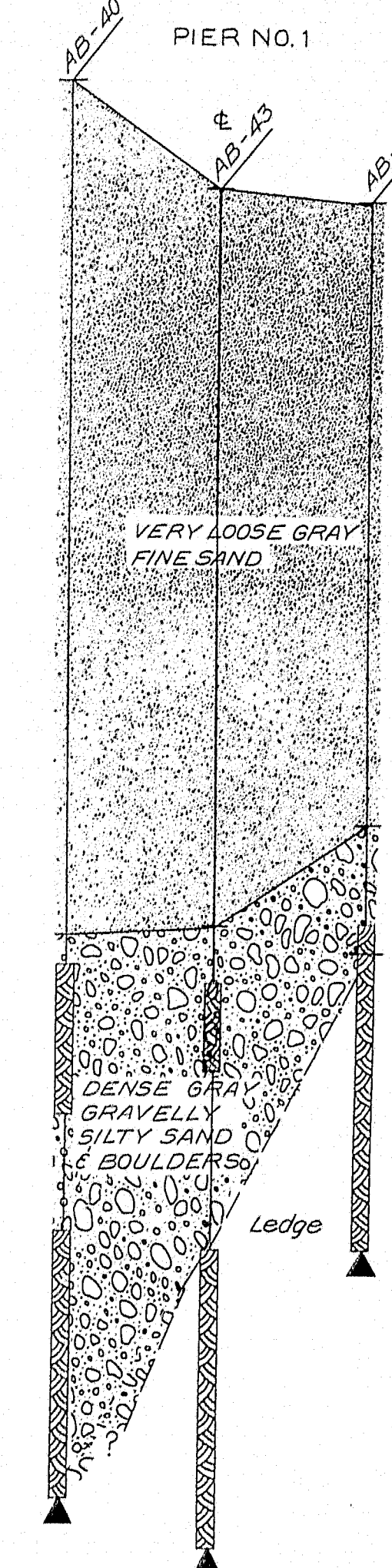
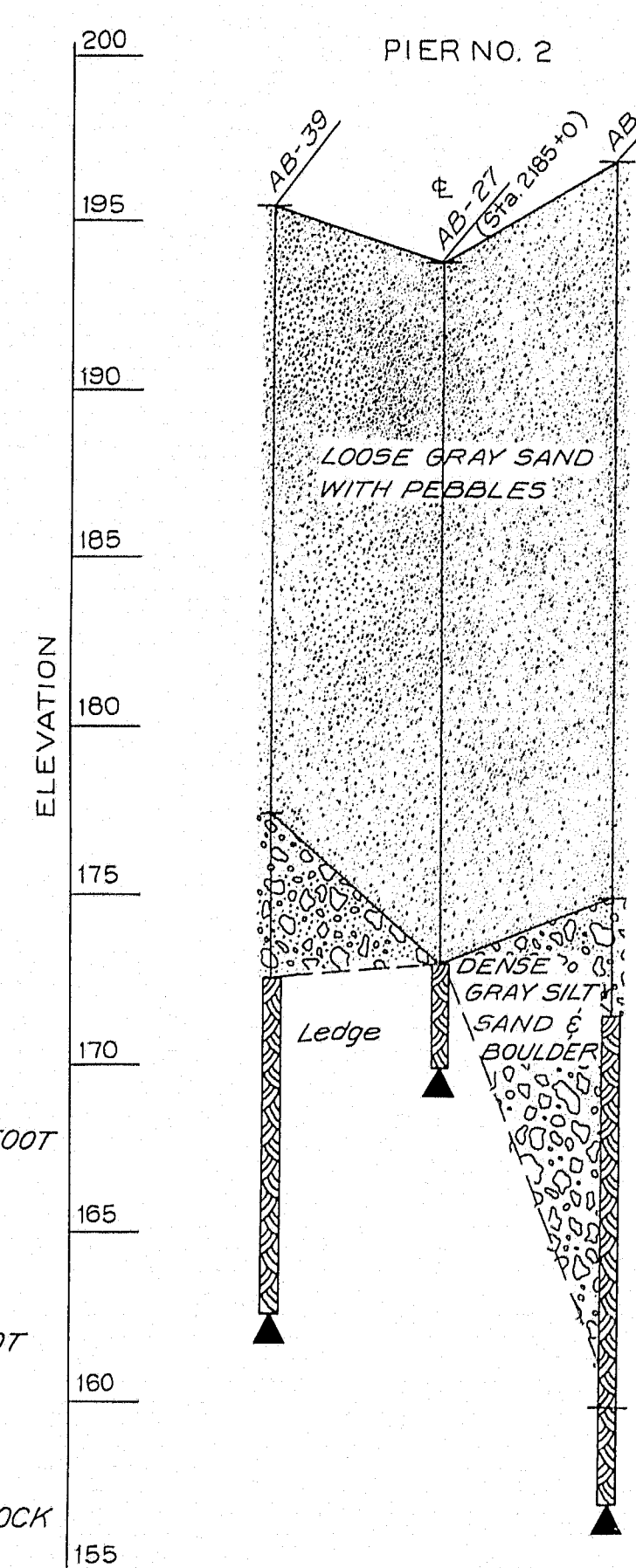


BORING AB-29 STATION 2186 +30 60' L.T. & S.B.



TRANSVERSE SECTIONS

(SOUTHBOUND)



- BORING NOTES**
- NUMBER OF BLOWS REQUIRED TO DRIVE EXTRA HEAVY CASING ONE FOOT WITH 400 FT. LBS. OF ENERGY PER BLOW
 - LOCATION OF SAMPLE OR SAMPLE ATTEMPT
 - S & H SAMPLER #1290
 - 3 1/2" O.D. 16 GA. SEAMLESS TUBING
 - UNSUCCESSFUL SAMPLE ATTEMPT AND TYPE OF SAMPLER
 - NUMBER OF BLOWS REQUIRED TO DRIVE SPOON OR TUBING ONE FOOT WITH 350 FT. LBS. OF ENERGY PER BLOW
 - PISTON SAMPLER
 - FIELD VANE TEST
 - BOTTOM OF BORING (MAY NOT BE BOTTOM OF SOIL STRATA)
 - LOCATIONS CORED BY DIAMOND BIT AND PER CENT RECOVERY OF ROCK
- SHEAR NOTES**
- FIELD VANE SHEAR STRENGTHS
 - LABORATORY VANE SHEAR STRENGTHS
- WATER CONTENT NOTES**
- NATURAL WATER CONTENTS, GIVEN AS PER CENT OF DRY WEIGHT
 - PLASTIC AND LIQUID LIMITS

DESIGN—
TRACE—
CHECK—

BRIDGE NO.
SURVEY—
PLOT—

STATE HIGHWAY COMMISSION
BRIDGE DIVISION

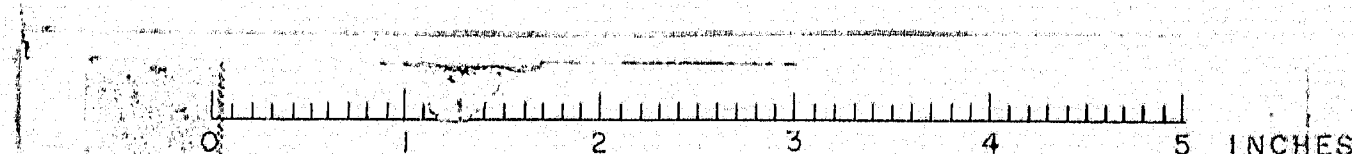
SEBASTICOOK RIVER BRIDGE

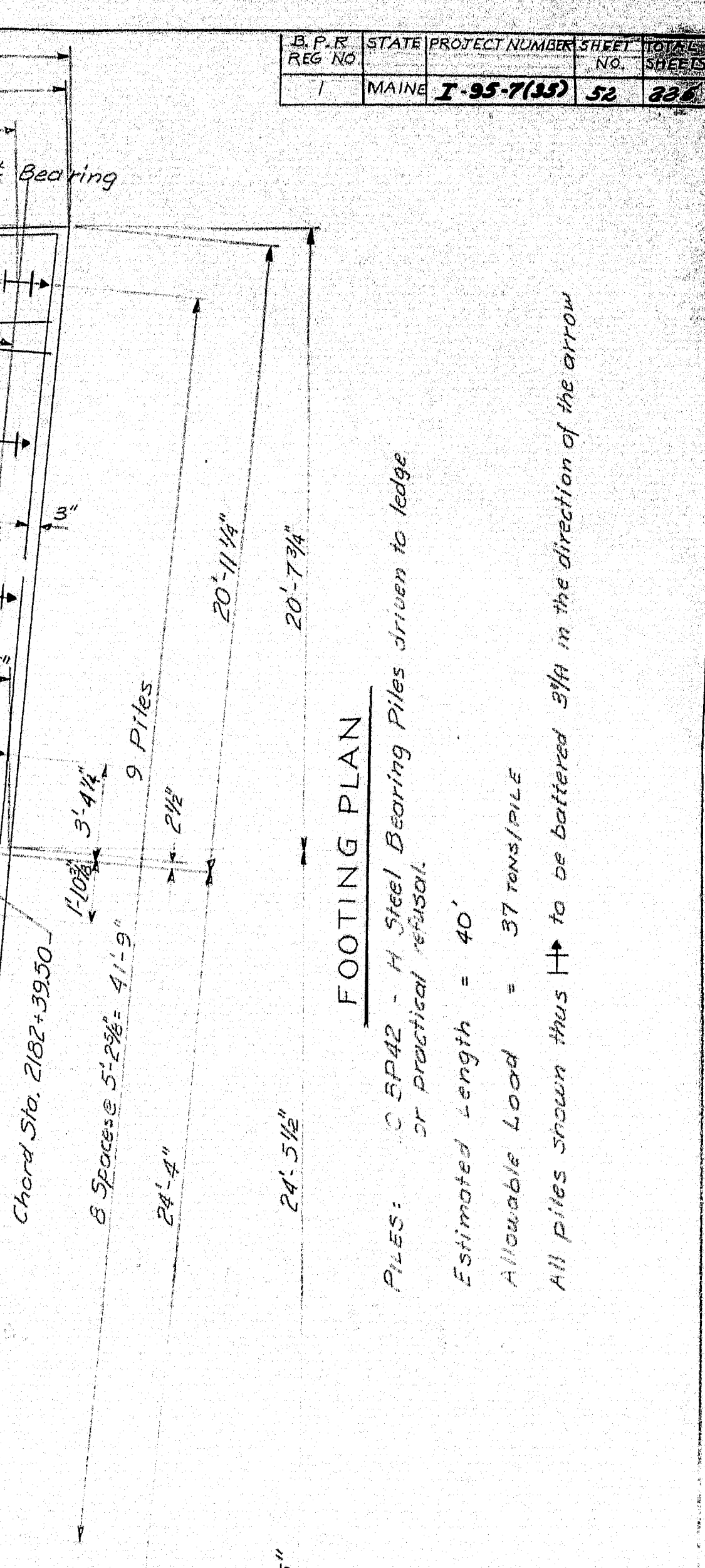
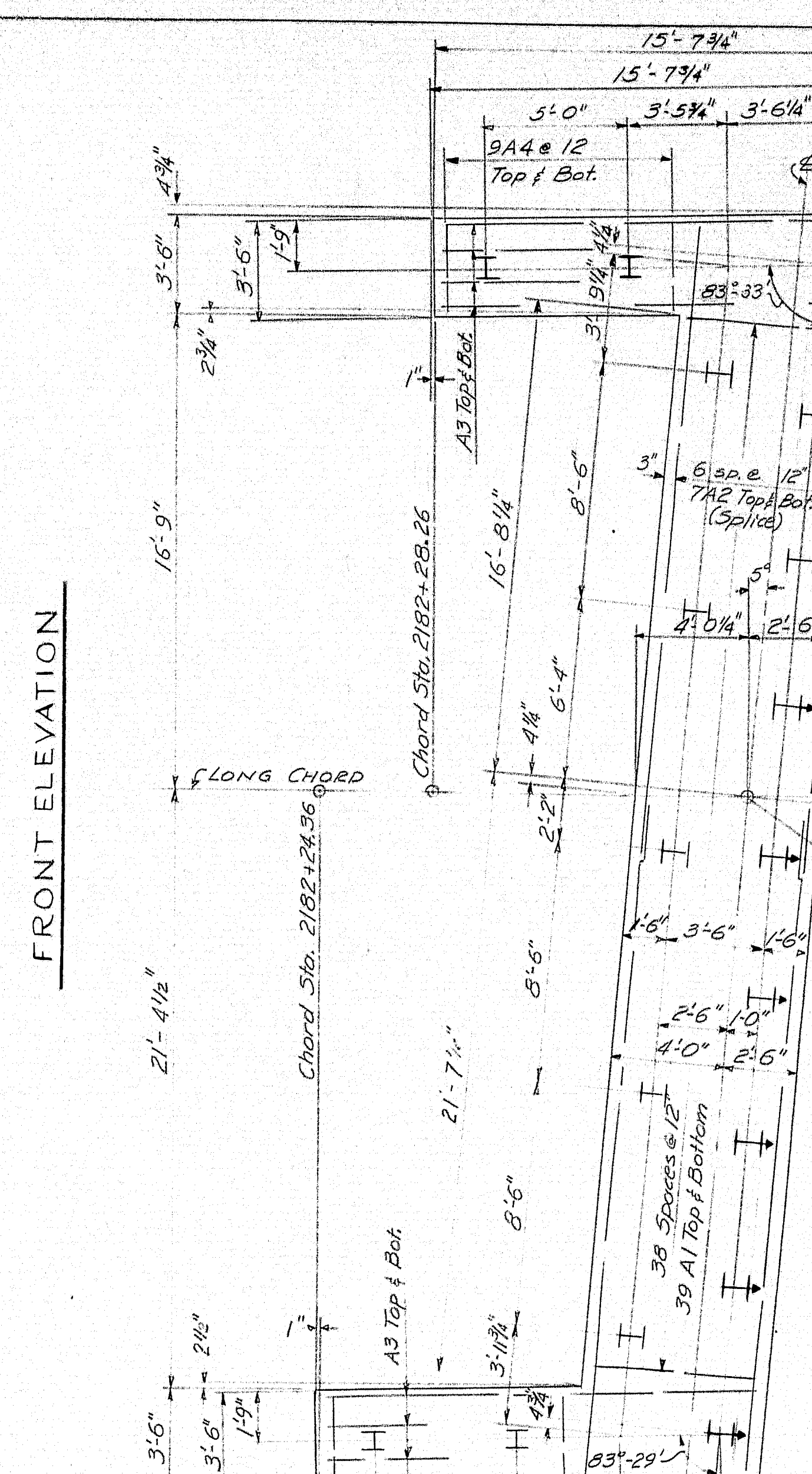
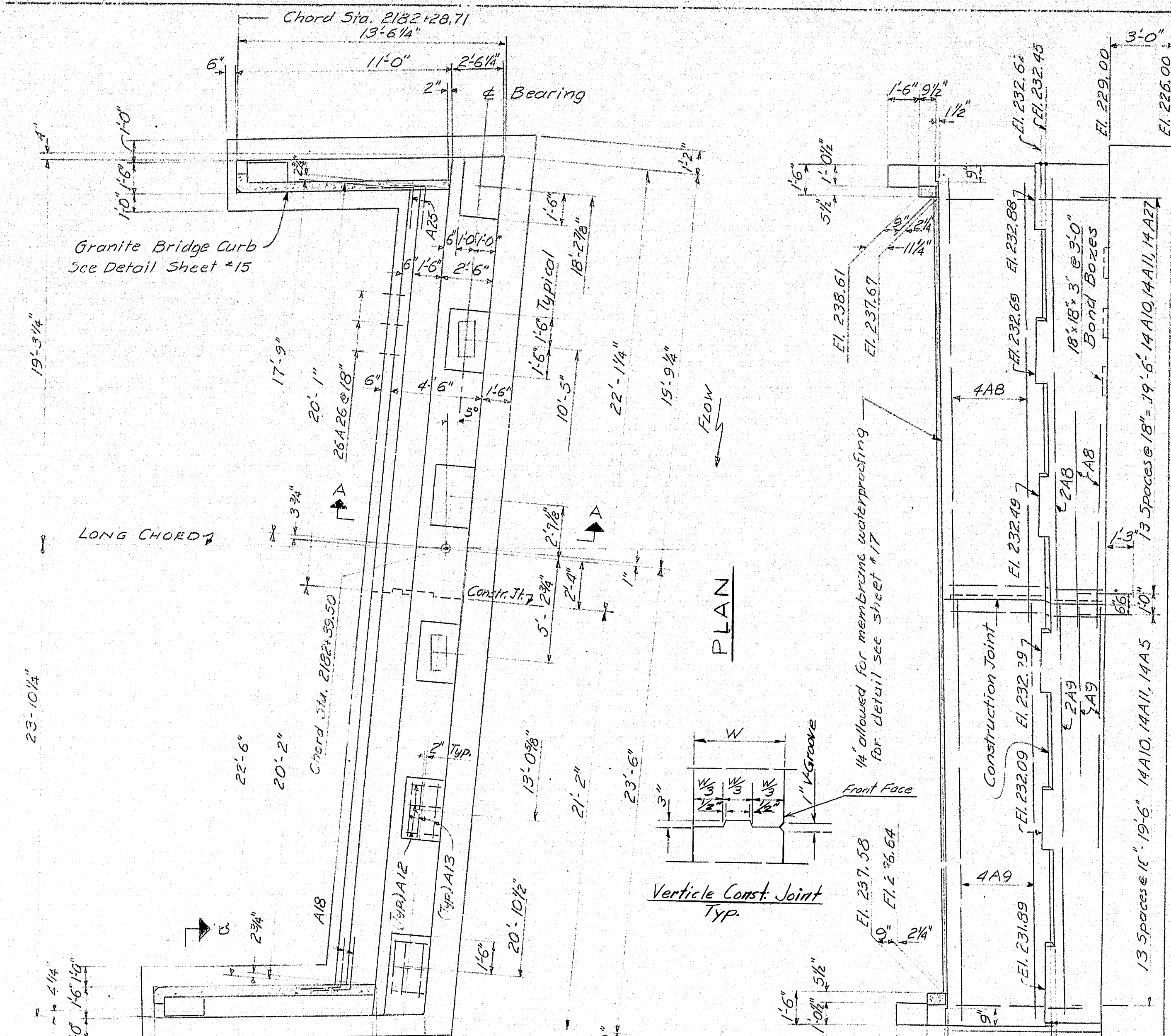
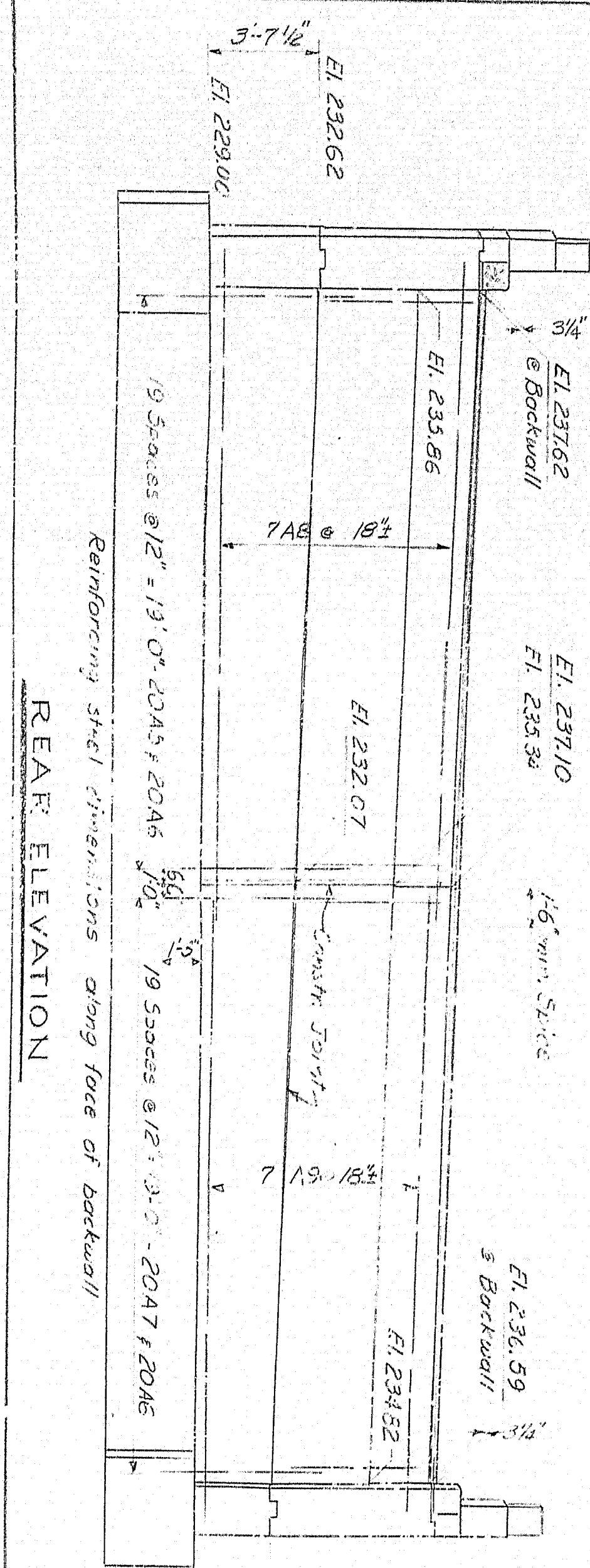
IN THE TOWN OF
PITTSFIELD
SOMERSET COUNTY

BORING DETAILS

SHEET 7 OF 18 AUGUSTA, MAINE

M-1986





GENERAL ABUTMENT NOTES

Dress bearing areas 1" larger all around than masonry plates, and to exact elevations shown. Caulk around edges of masonry plates with an approved caulking material. Payment to be incidental to contract items. Paint bridge seat, face of backwall, and 10' low top of riprap on face of breast wall with Gray Epoxy Resin Surface Sealant.

Gravel borrow shall be placed up to the bottom of the footing elevation before piles are driven.

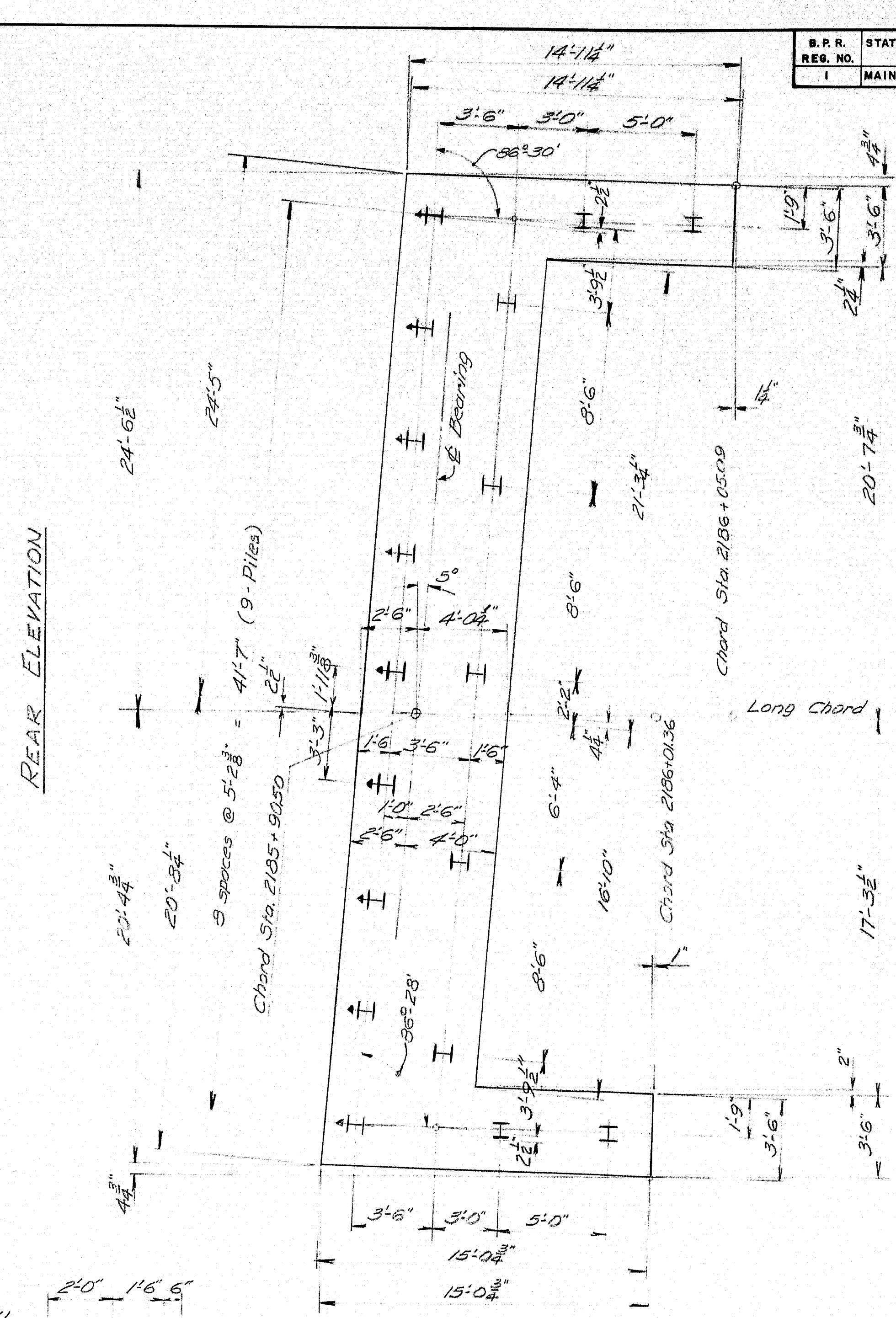
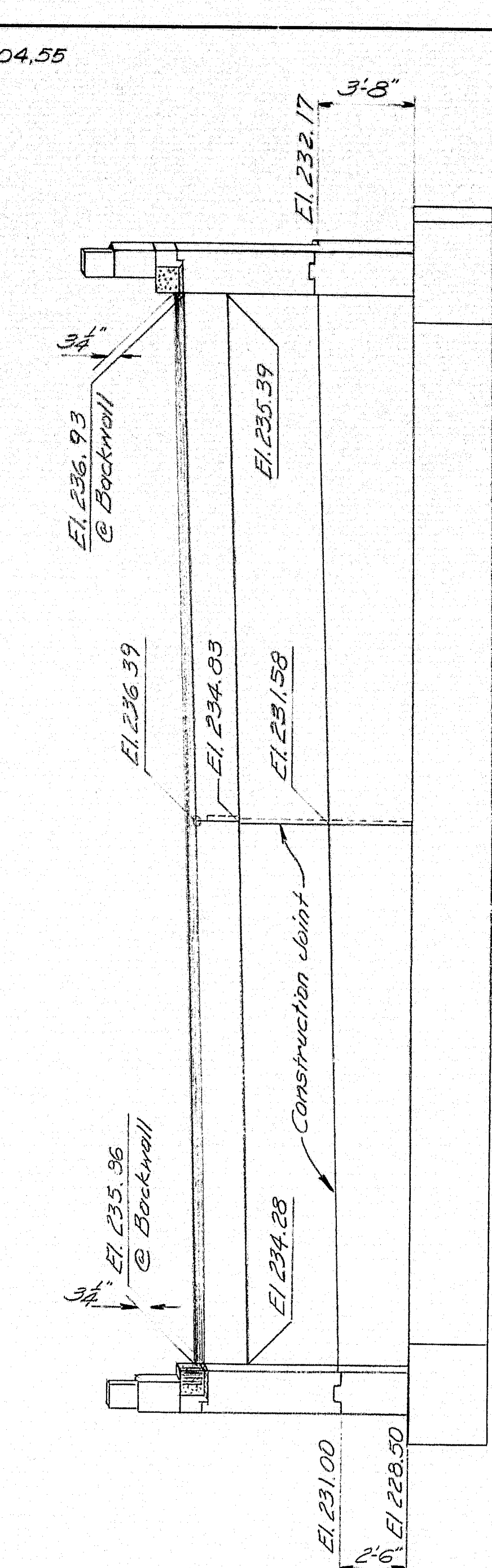
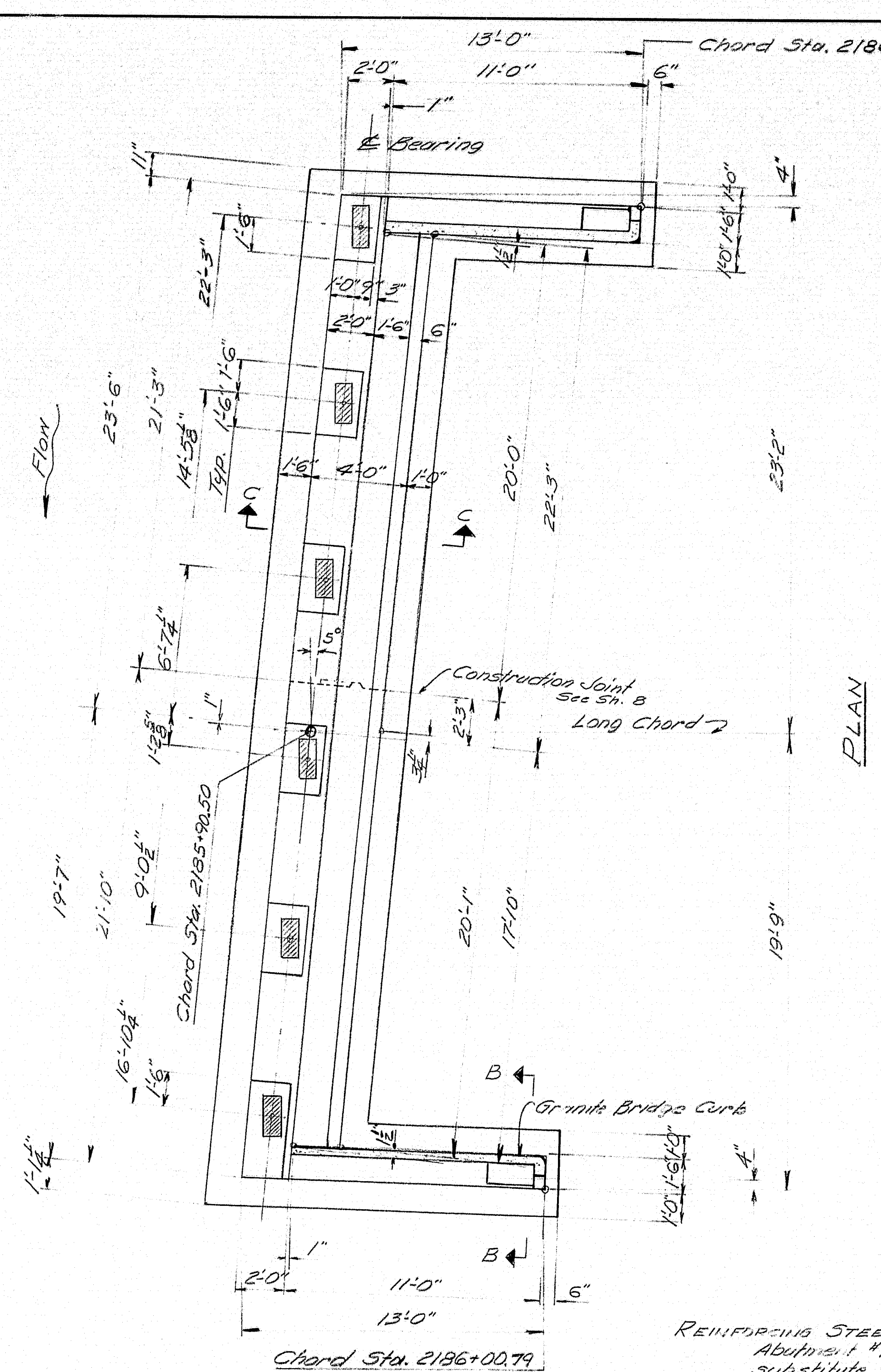
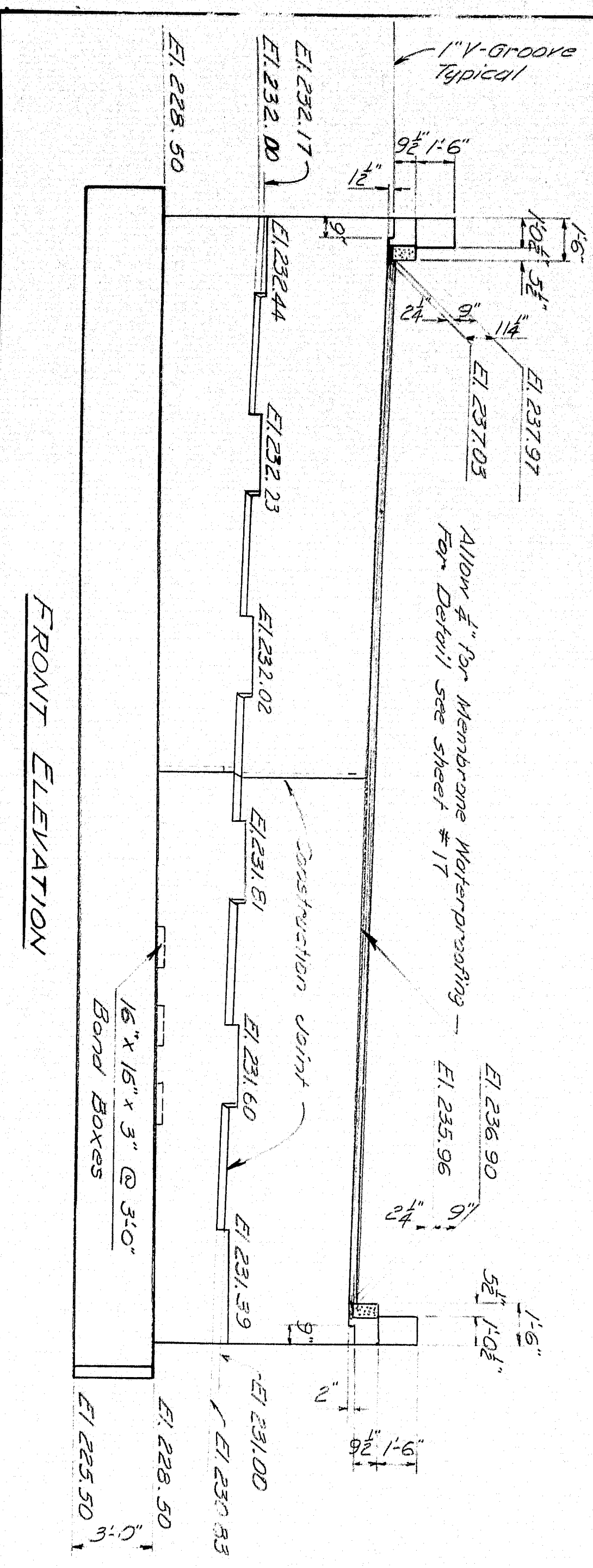
Cover all vertical construction joints on the back with two layers of heavy roofing 10' wide. Coat the concrete and the back of each layer of roofing, as applied, with a suitable grade of plastic cement. Recess area to be covered 1/4".

All reinforcing steel to be 2" clear unless otherwise shown.

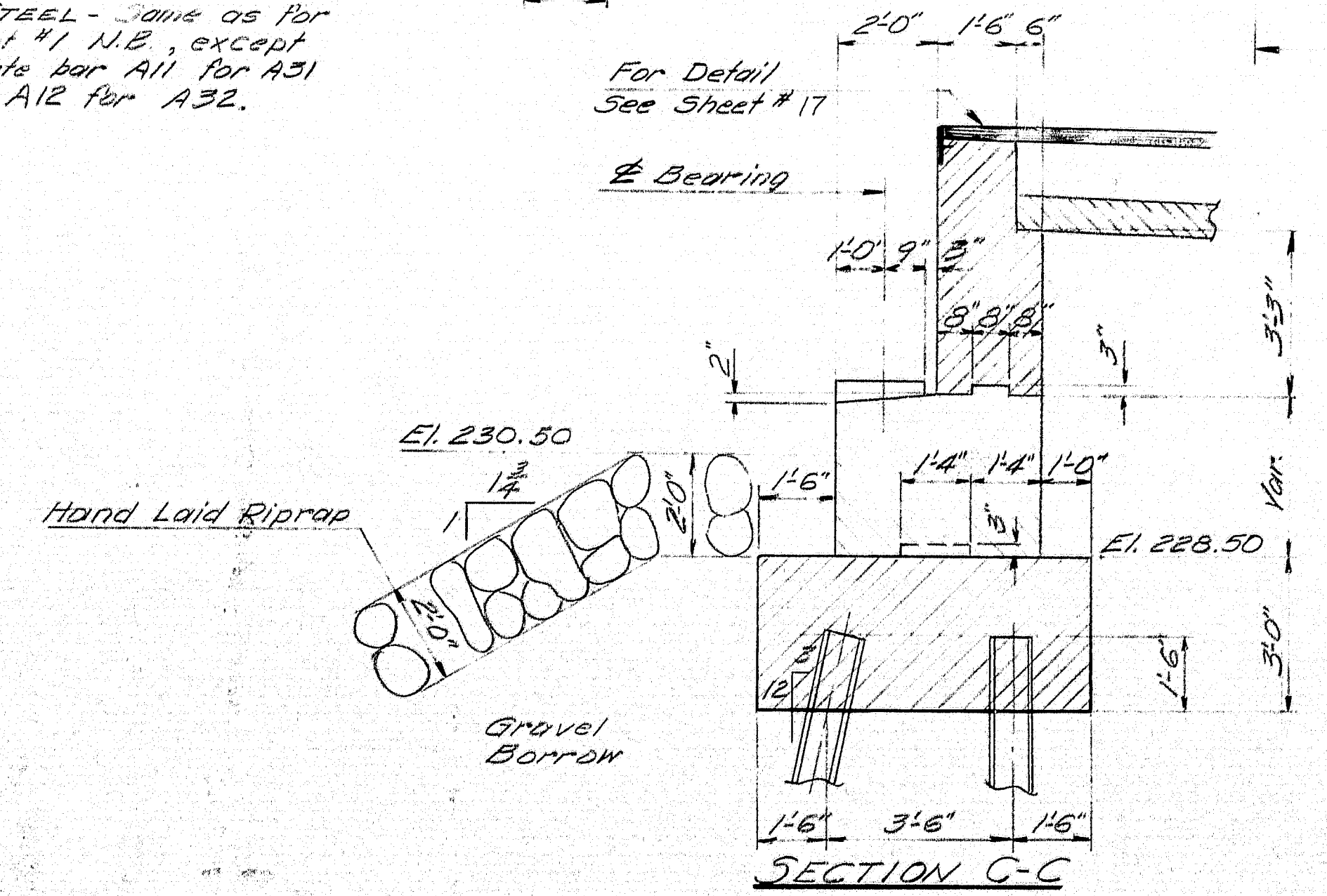
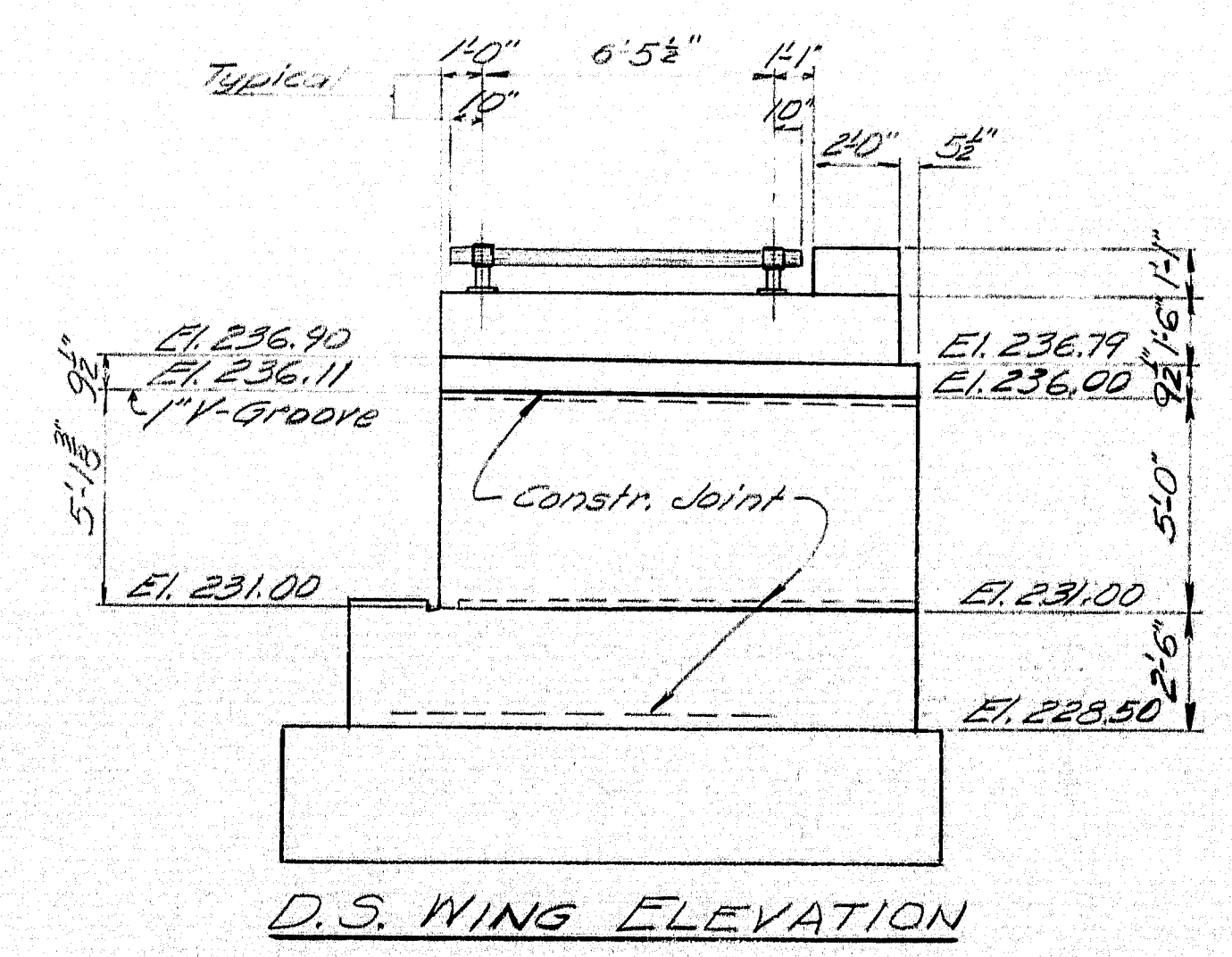
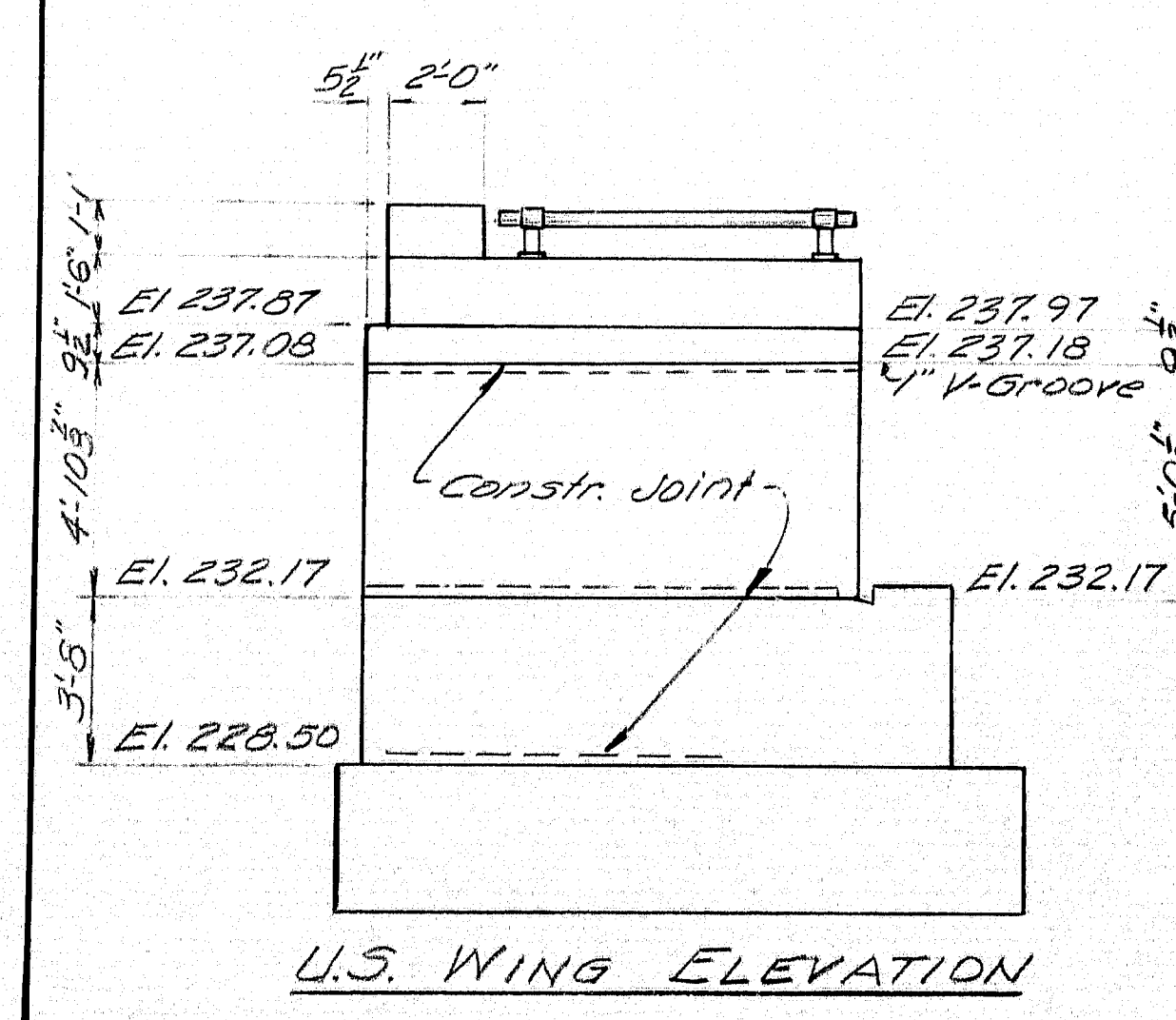
DESIGN T.H.K.	CHECK T.H.K.
STATE HIGHWAY COMMISSION BRIDGE DIVISION	
SEBASTICOOK RIVER BRIDGE	
IN THE TOWN OF PITTSFIELD	
SOMERSET COUNTY	
ABUTMENT 1 N.B.	
SHEET 8 OF 18 AUGUSTA, MAINE FEB 1963	

M-1937

S. P. R.	STATE	PROJECT NUMBER	SHEET	TOTAL
REG. NO.	MAINE	1-95-7 (35)	55	225



PILES: 10 BD 42 - H Steel Bearing Piles driven to ledge or practical refusal.
 Estimated Length = 62'
 Allowable Load = 37 tons/pile
 All piles shown thus H+ to be battered 3 3/4\"/>



DESIGN - T.H.K. DETAIL - R.D.
 TRACE - G.M.C.
 CHECK - A.R.S.

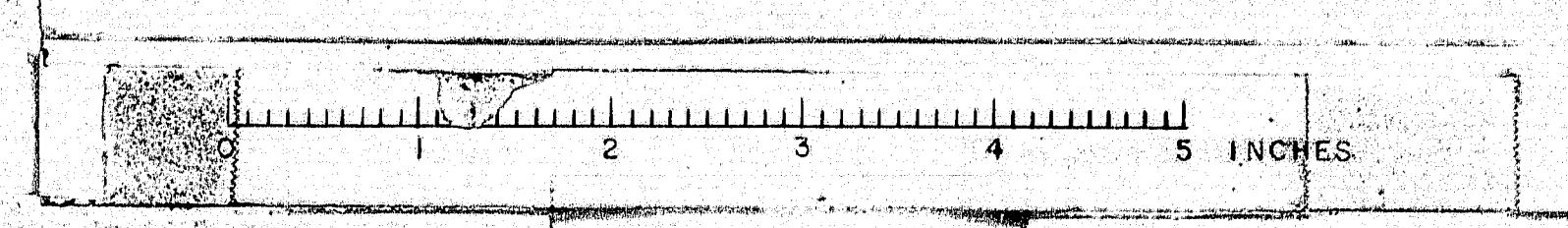
STATE HIGHWAY COMMISSION
 BRIDGE DIVISION

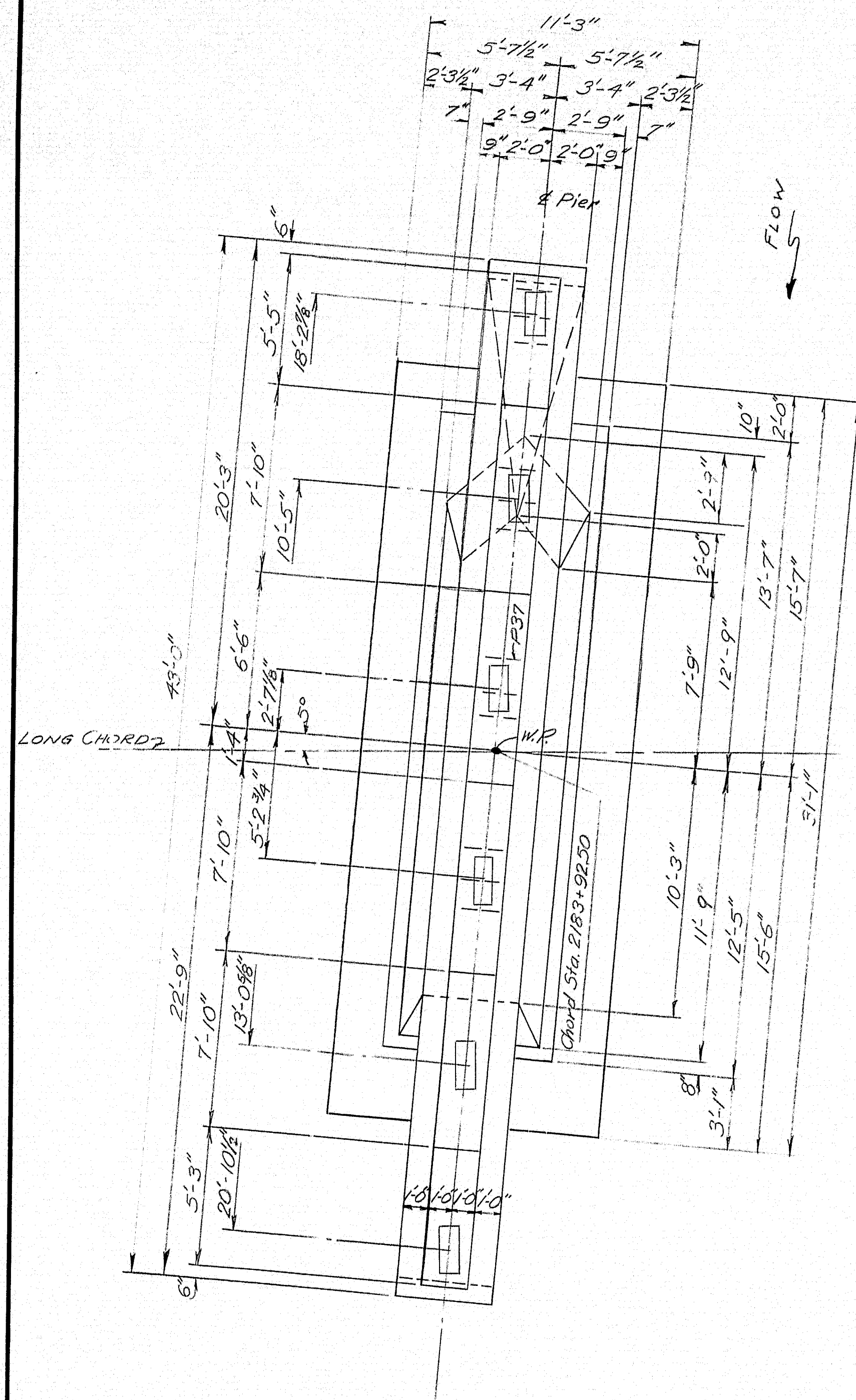
SEBASTICOOK RIVER BRIDGE

IN THE TOWN OF
PITTSFIELD
SOMERSET COUNTY

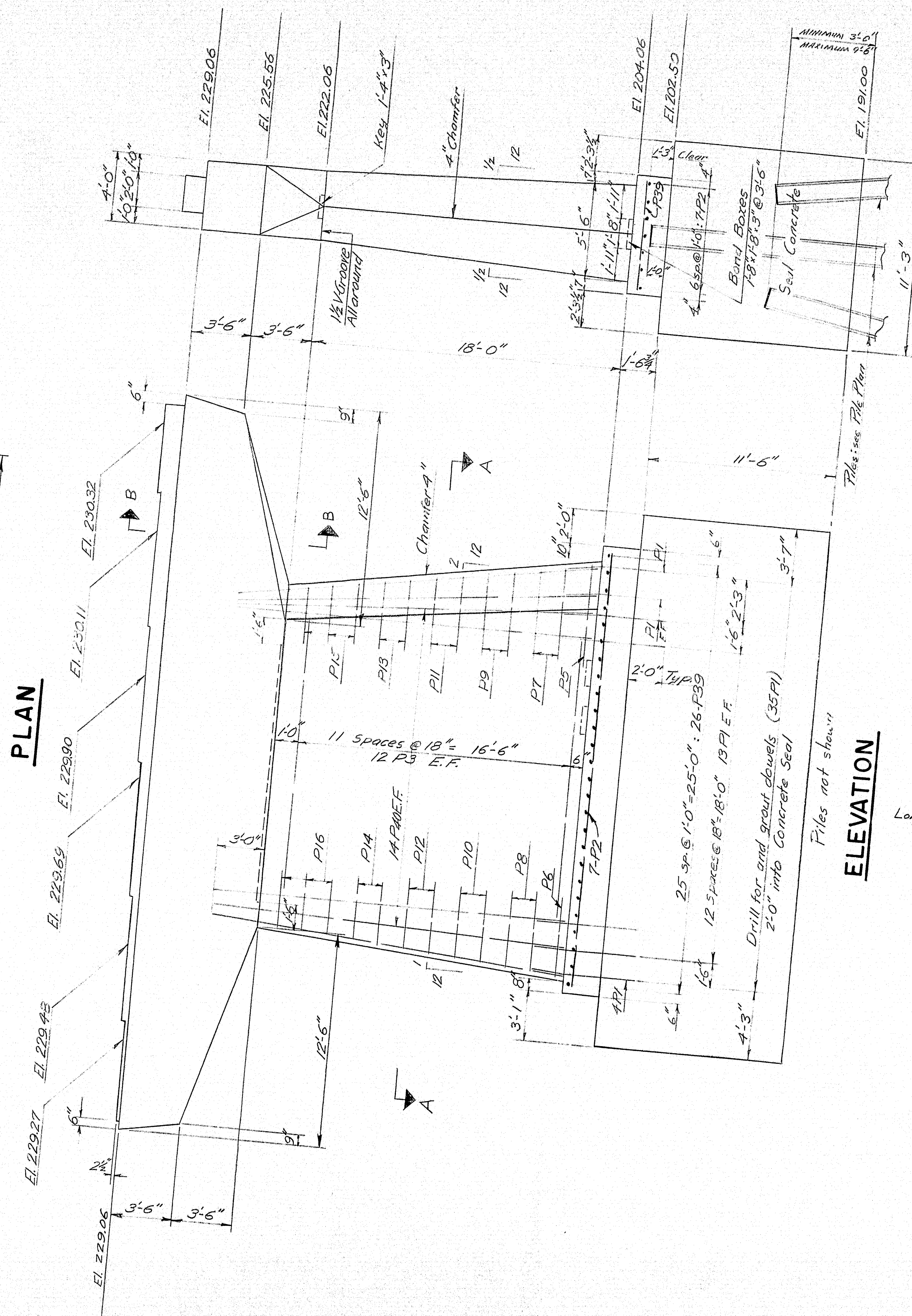
ABUTMENT NO. 2, SOUTHBOUND

SHEET 11 OF 18 AUGUSTA, MAINE FEB 1963





PLAN



SIDE ELEVATION

ELEVATION

SEAL & PILE PLAN

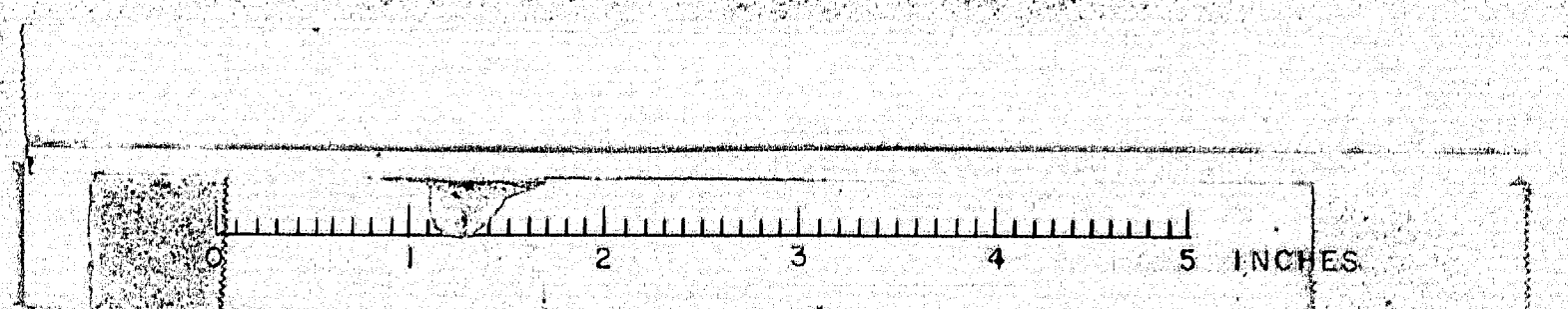
PILE NOTES

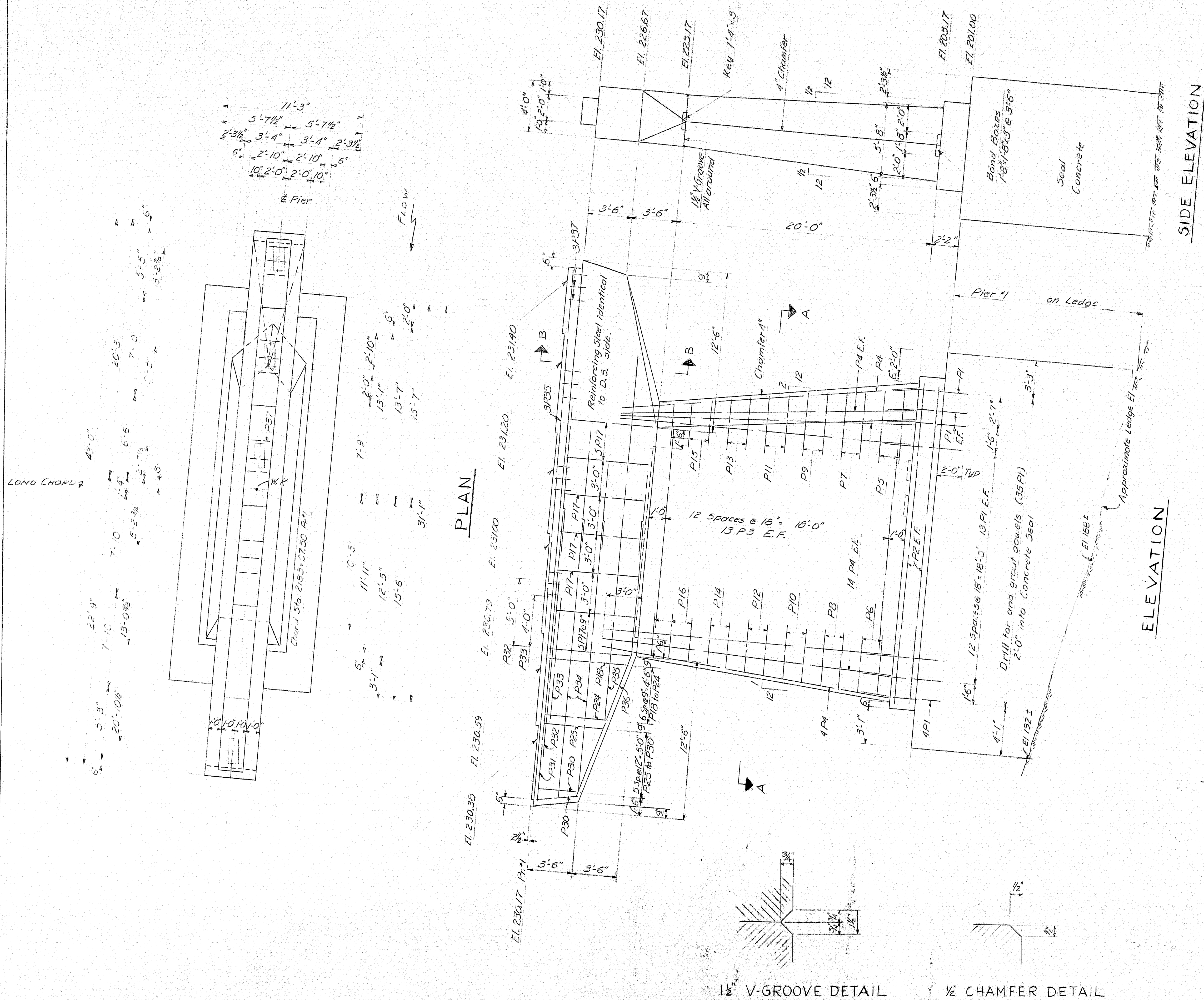
1. Piles are 14 BP73-H steel bearing piles driven to ledge or practical refusal.
2. Estimated length:
4 piles @ 30'
14 piles @ 25'
3. Allowable load: 64.4 tons per pile.
4. All piles shown thus \rightarrow shall be battered two (2) inches per foot in the direction of the arrow.
5. Pile spacing dimensions apply at bottom of seal, El. 191.00.
6. Inner piles only (those circled on Plan) have tops at El. 203.50

DESIGN - TKM	DET. - RWL	BRIDGE NO.
TRACE - RWL	SURVEY -	
CHECK - TKM	PLOT -	
STATE HIGHWAY COMMISSION BRIDGE DIVISION		
SEBASTICOOK RIVER BRIDGE		
IN THE TOWN OF PITTSFIELD		
SOMERSET COUNTY		
PIER 2 NB		
SHEET 12A OF 18	AUGUSTA, MAINE	FEB 1963

For Sect. A-R, Sect. B-F, & General Pier Notes see Sh. #12

M-1991





GENERAL PIER NOTES:

Dress bearing areas larger all around than masonry plates, and to exact elevations shown. Caulk around edges of masonry plates with an approved caulking material. Payment to be incidental to contract items.

Seal concrete dimensions are given predicated on use of MP-116, DP2 I-27 or equivalent steel sheet piling with appropriate standard rolled corners. Pay dimensions for seal concrete shall be neat dimensions as shown, plus ten (10) inches.

The depths of the concrete seals have been calculated assuming a water elevation of 216.0 and bottom of footings at elevations shown. Seal concrete is intended to be placed under water and to be paid for under Item 701-36.

Payment for drilling and grouting dowels into concrete seal shall be incidental to Items 705-13 and 705-14.

Piers were designed for a six inch thickness of ice applied at elevation 219.0 and a stream flow of 5 Msec.

All reinforcing steel to be 5 inches clear, unless otherwise shown.

Chamfer all exposed edges 1/2", unless otherwise noted.

E.F. = Each Face

DESIGN T.H.K.
DETAIL T.H.K.
CHECK R.R.S.

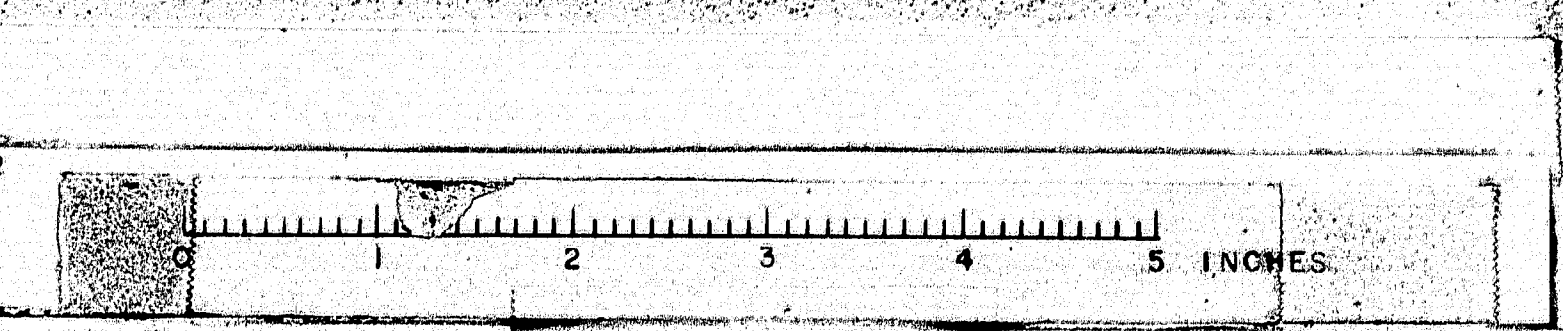
STATE HIGHWAY COMMISSION
BRIDGE DIVISION

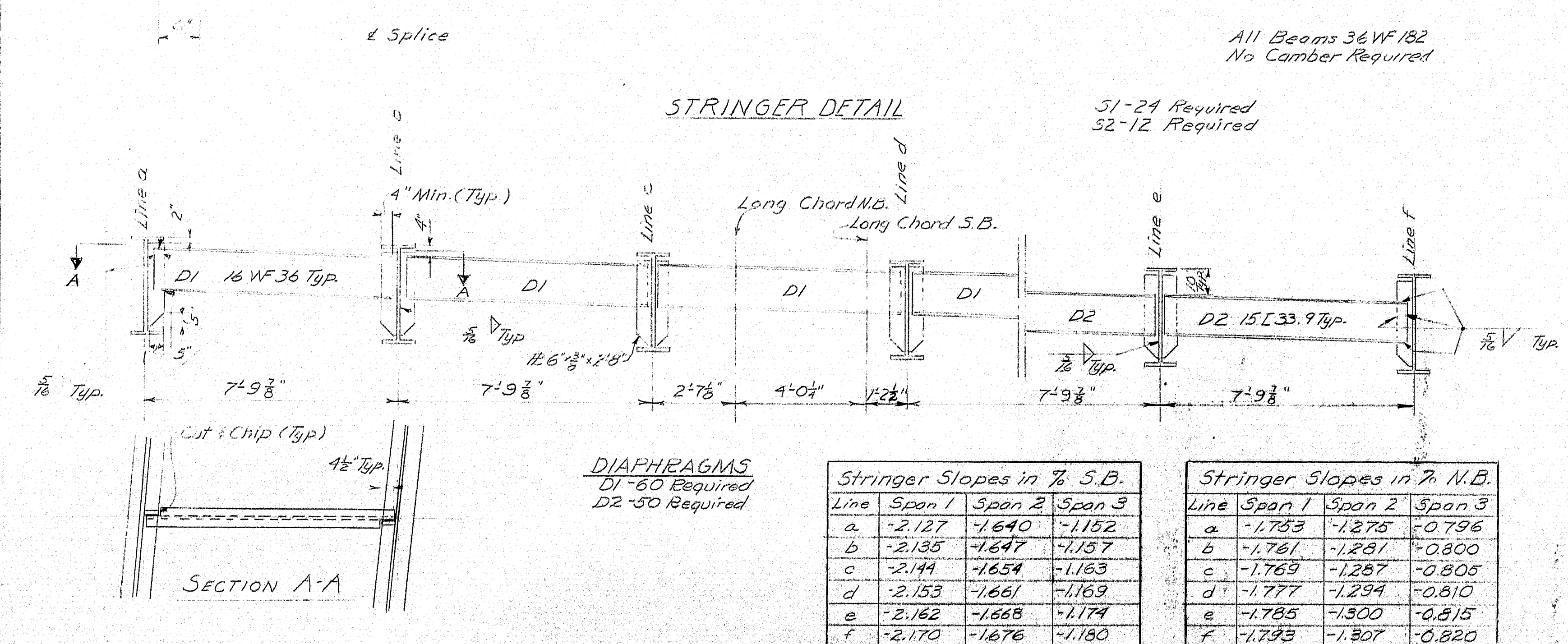
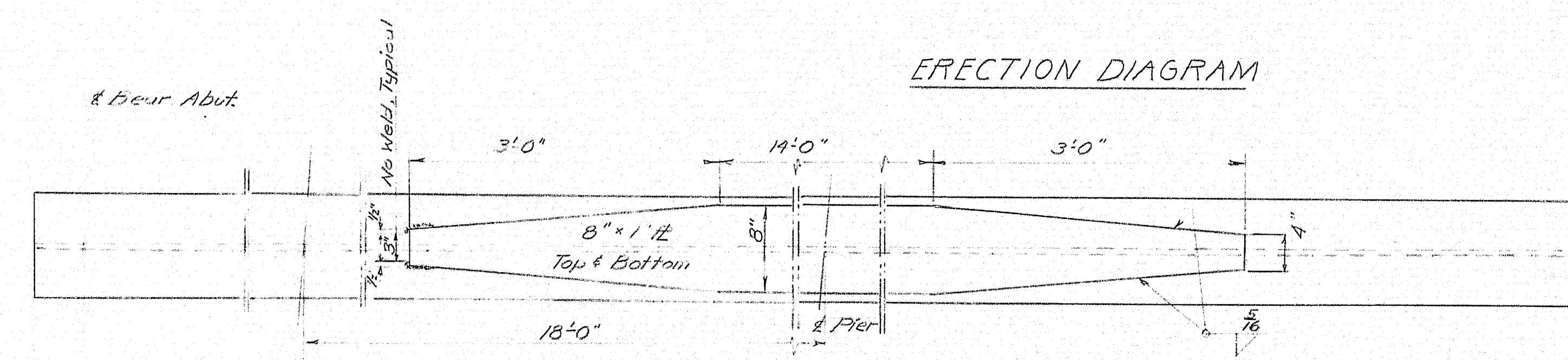
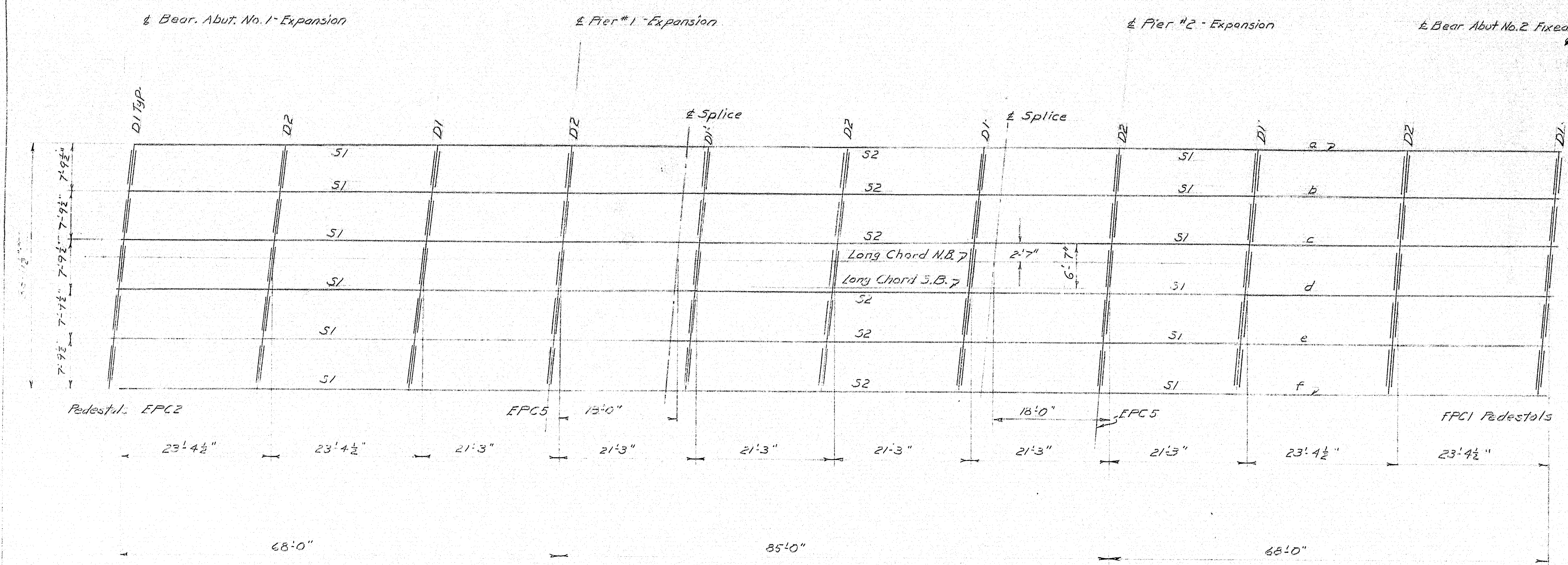
SEBASTICOOK RIVER BRIDGE

IN THE TOWN OF
PITTSFIELD
SOMERSET COUNTY

PIER 1 N.B.

SHEET 12 OF 18 AUGUSTA, MAINE FEB. 1963

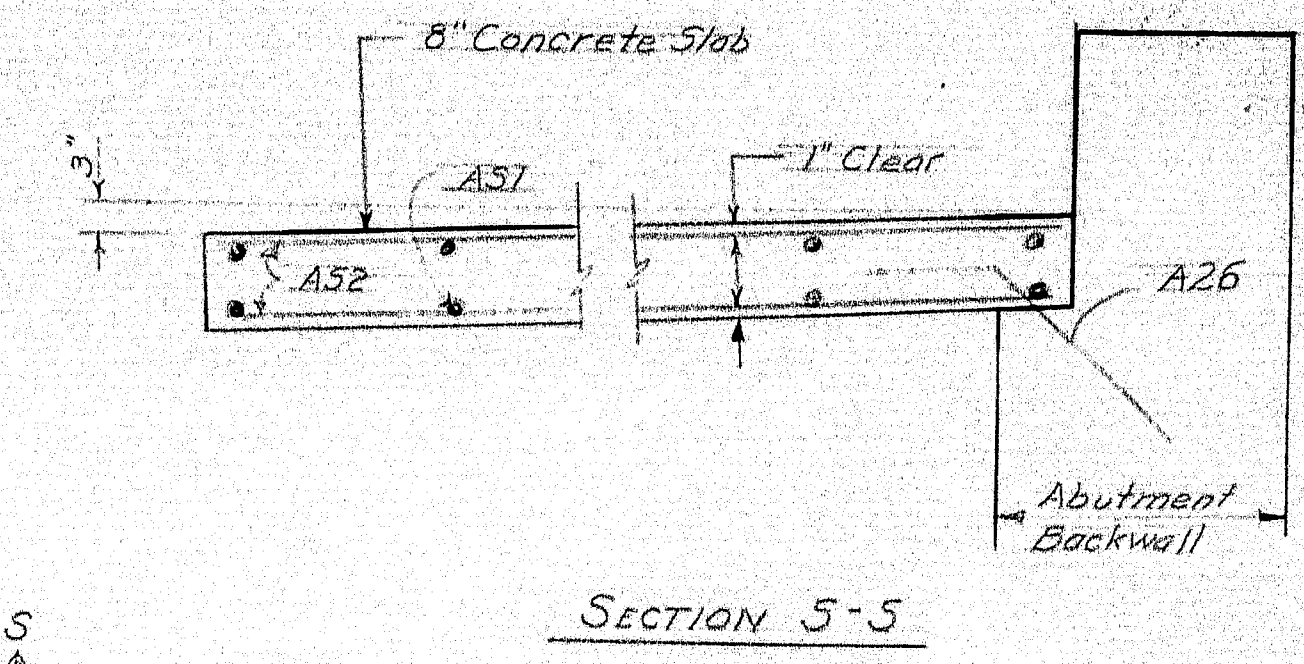
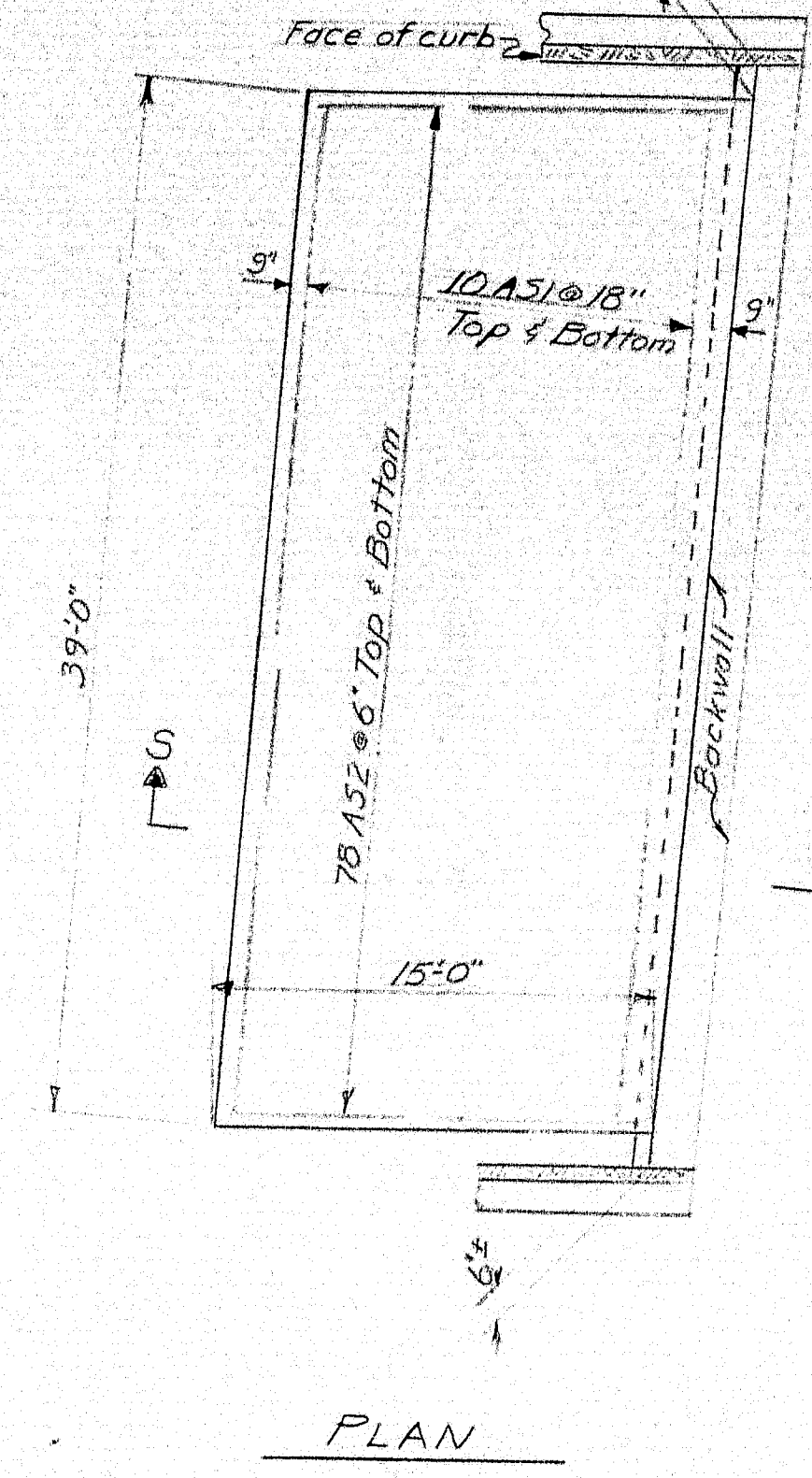
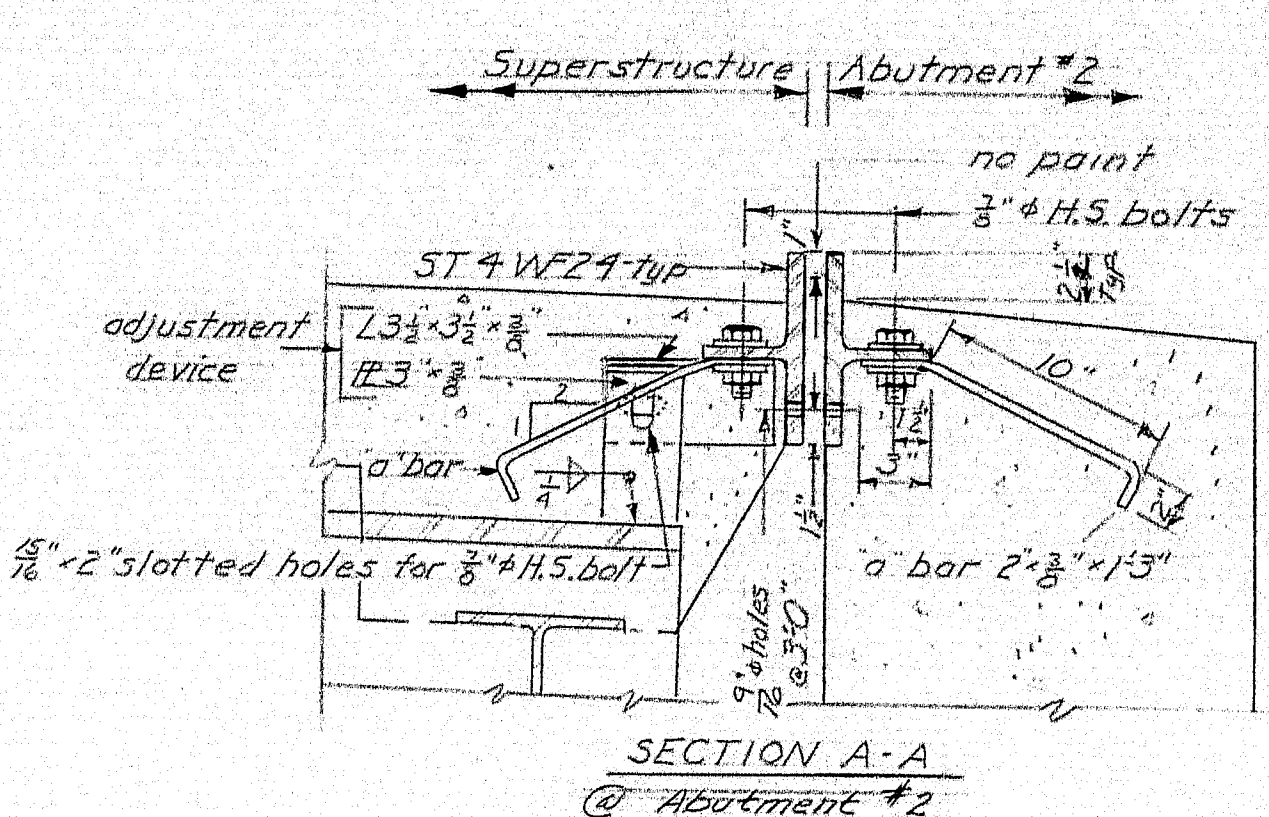
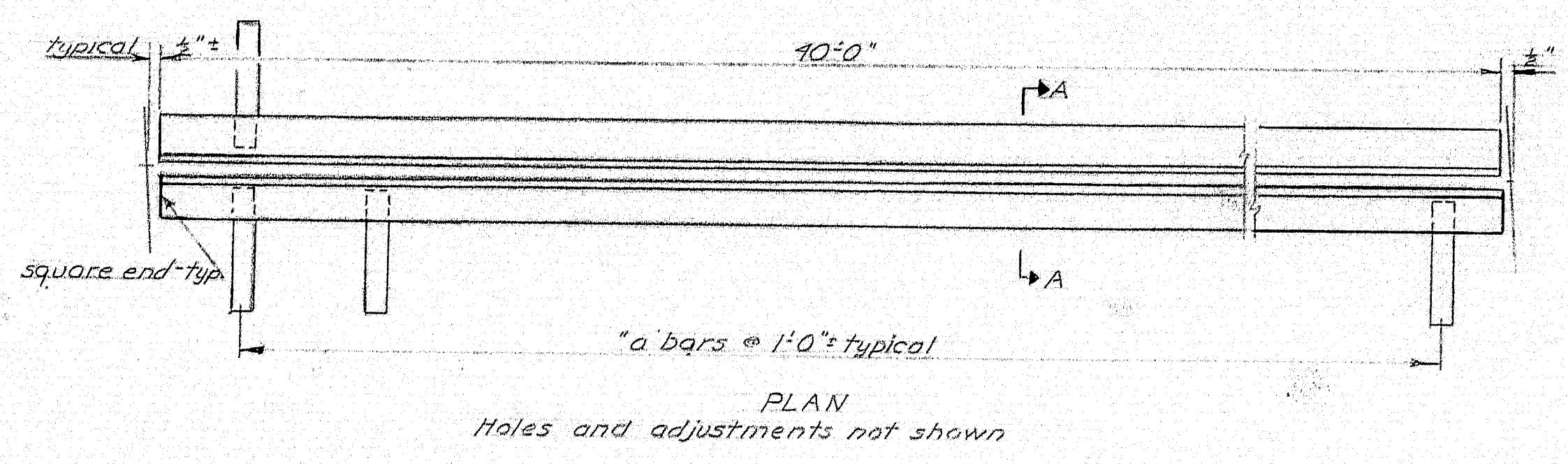




Stringer Slopes in % S.B.			
Line	Span 1	Span 2	Span 3
a	-2.127	-1.640	-1.152
b	-2.135	-1.647	-1.157
c	-2.144	-1.654	-1.163
d	-2.153	-1.661	-1.169
e	-2.162	-1.668	-1.174
f	-2.170	-1.676	-1.180

Stringer Slopes in % N.B.			
Line	Span 1	Span 2	Span 3
a	-1.783	-1.275	-0.796
b	-1.781	-1.281	-0.800
c	-1.769	-1.287	-0.805
d	-1.777	-1.294	-0.810
e	-1.785	-1.300	-0.815
f	-1.793	-1.307	-0.820

BLOCKING TABLE										
Span	point	elevation point L see Sect. 6.6	bottom of slab grades above points on beams indicated						elevation point R see Sect. 6.6	
			a	b	c	d	e	f		
SOUTHBOUND	1	0	240.07	240.04	239.80	239.70	239.57	239.31	239.12	239.00
		$\frac{1}{4}$	239.74	239.75	239.55	239.35	239.16	238.86	238.76	238.70
		$\frac{1}{2}$	239.39	239.39	239.19	238.99	238.79	238.54	238.34	238.34
		$\frac{3}{4}$	239.01	239.00	238.80	238.60	238.40	238.20	238.01	237.97
		1	238.67	238.65	238.45	238.24	238.04	237.84	237.64	237.61
		$\frac{1}{2}$	238.37	238.35	238.15	237.94	237.74	237.54	237.34	237.31
	2	0	238.11	238.09	237.89	237.68	237.48	237.28	237.04	237.04
		$\frac{1}{4}$	237.83	237.81	237.61	237.40	237.20	237.00	236.74	236.76
		$\frac{1}{2}$	237.54	237.52	237.31	237.10	236.90	236.70	236.46	236.46
		$\frac{3}{4}$	237.27	237.25	237.04	236.83	236.63	236.43	236.19	236.19
		1	237.04	237.05	236.84	236.63	236.43	236.23	236.04	235.97
		$\frac{1}{4}$	236.79	236.80	236.59	236.38	236.18	235.98	235.74	235.78
3	0	236.47	236.48	236.27	236.06	235.86	235.66	235.43	235.57	
	$\frac{1}{4}$	236.16	236.17	235.96	235.75	235.55	235.35	235.11	235.31	
	$\frac{1}{2}$	235.81	235.82	235.61	235.40	235.20	235.00	234.74	234.81	
	$\frac{3}{4}$	235.51	235.52	235.31	235.10	234.90	234.70	234.46	234.57	
	1	235.24	235.25	235.04	234.83	234.63	234.43	234.19	234.31	
	$\frac{1}{4}$	234.94	234.95	234.74	234.53	234.33	234.13	233.89	234.01	
4	0	234.67	234.68	234.47	234.26	234.06	233.86	233.62	233.74	
	$\frac{1}{4}$	234.37	234.38	234.17	233.96	233.76	233.56	233.32	233.44	
	$\frac{1}{2}$	234.07	234.08	233.87	233.66	233.46	233.26	233.02	233.14	
	$\frac{3}{4}$	233.77	233.78	233.57	233.36	233.16	232.96	232.72	232.84	
	1	233.50	233.51	233.30	233.09	232.89	232.69	232.45	232.57	
	$\frac{1}{4}$	233.20	233.21	233.00	232.79	232.59	232.39	232.15	232.27	
5	0	232.94	232.95	232.74	232.53	232.33	232.13	231.89	232.01	
	$\frac{1}{4}$	232.64	232.65	232.44	232.23	232.03	231.83	231.59	231.71	
	$\frac{1}{2}$	232.34	232.35	232.14	231.93	231.73	231.53	231.29	231.41	
	$\frac{3}{4}$	232.04	232.05	231.84	231.63	231.43	231.23	230.99	231.11	
	1	231.77	231.78	231.57	231.36	231.16	230.96	230.72	230.84	
	$\frac{1}{4}$	231.47	231.48	231.27	231.06	230.86	230.66	230.42	230.54	
6	0	231.20	231.21	231.00	230.79	230.59	230.39	230.15	230.27	
	$\frac{1}{4}$	230.90	230.91	230.70	230.49	230.29	230.09	229.85	229.97	
	$\frac{1}{2}$	230.60	230.61	230.40	230.19	229.99	229.79	229.55	229.67	
	$\frac{3}{4}$	230.30	230.31	230.10	229.89	229.69	229.49	229.25	229.37	
	1	230.03	230.04	229.82	229.61	229.41	229.21	228.97	229.09	
	$\frac{1}{4}$	229.73	229.74	229.52	229.31	229.11	228.91	228.67	228.79	
7	0	229.46	229.47	229.26	229.05	228.85	228.65	228.41	228.53	
	$\frac{1}{4}$	229.16	229.17	228.95	228.74	228.54	228.34	228.10	228.22	
	$\frac{1}{2}$	228.86	228.87	228.65	228.44	228.24	228.04	227.80	227.92	
	$\frac{3}{4}$	228.56	228.57	228.35	228.14	227.94	227.74	227.50	227.62	
	1	228.29	228.30	228.08	227.87	227.67	227.47	227.23	227.35	
	$\frac{1}{4}$	227.99	228.00	227.78	227.57	227.37	227.17	226.93	227.05	
8	0	227.72	227.73	227.51	227.30	227.10	226.90	226.66	226.78	
	$\frac{1}{4}$	227.42	227.43	227.21	227.00	226.80	226.60	226.36	226.48	
	$\frac{1}{2}$	227.12	227.13	226.91	226.70	226.50	226.30	226.06	226.18	
	$\frac{3}{4}$	226.82	226.83	226.61	226.40	226.20	226.00	225.76	225.88	
	1	226.55	226.56	226.34	226.13	225.93	225.73	225.49	225.61	
	$\frac{1}{4}$	226.25	226.26	226.04	225.83	225.63	225.43	225.19	225.31	
9	0	225.98	225.99	225.77	225.56	225.36	225.16	224.92	225.04	
	$\frac{1}{4}$	225.68	225.69	225.47	225.26	225.06	224.86	224.62	224.74	
	$\frac{1}{2}$	225.38	225.39	225.17	224.96	224.76	224.56	224.32	224.44	
	$\frac{3}{4}$	225.08	225.09	224.87	224.66	224.46	224.26	224.02	224.14	
	1	224.81	224.82	224.60	224.39	224.19	223.99	223.75	223.87	
	$\frac{1}{4}$	224.51	224.52	224.30	224.09	223.89	223.69	223.45	223.57	
10	0	224.24	224.25	224.03	223.82	223.62	223.42	223.18	223.30	
	$\frac{1}{4}$	223.94	223.95	223.73	223.52	223.32	223.12	222.88	223.00	
	$\frac{1}{2}$	223.64	223.65	223.43	223.22	223.02	222.82	222.58	222.70	
	$\frac{3}{4}$	223.34	223.35	223.13	222.92	222.72	222.52	222.28	222.40	
	1	223.07	223.08	222.85	222.64	222.44	222.24	222.00	222.12	
	$\frac{1}{4}$	222.77	222.78	222.56	222.35	222.15	221.95	221.71	221.83	
11	0	222.50	222.51	222.28	222.07	221.87	221.67	221.43	221.55	
	$\frac{1}{4}$	222.20	222.21	221.99	221.78	221.58	221.38	221.14	221.26	
	$\frac{1}{2}$	221.90	221.91	221.69	221.48	221.28	221.08	220.84	220.96	
	$\frac{3}{4}$	221.60	221.61	221.39	221.18	220.98	220.78	220.54	220.66	
	1	221.33	221.34	221.11	220.90	220.70	220.50	220.26	220.38	
	$\frac{1}{4}$	221.03	221.04	220.82	220.61	220.41	220.21	219.97	220.09	
12	0	220.76	220.77	220.54	220.33	220.13	219.93	219.69	219.81	
	$\frac{1}{4}$	220.46	220.47	220.24	220.03	219.83	219.63	219.39	219.51	
	$\frac{1}{2}$	220.16	220.17	219.94	219.73	219.53	219.33	219.09	219.21	
	$\frac{3}{4}$	219.86	219.87	219.64	219.43	219.23	219.03	218.79	218.91	
	1	219.59	219.60	219.37	219.16	218.96	218.76	218.52	218.64	
	$\frac{1}{4}$	219.29	219.30	219.07	218.86	218.66	218.46	218.22	218.34	
13	0	219.02	219.03	218.80	218.59	218.39	218.19	217.95	218.07	
	$\frac{1}{4}$	218.72	218.73	218.50	218.29	218.09	217.89	217.65	217.77	
	$\frac{1}{2}$	218.42	218.43	218.20	217.99	217.79	217.59	217.35	217.47	
	$\frac{3}{4}$	218.12	218.13	217.90	217.69	217.49	217.29	217.05	217.17	
	1	217.85	217.86	217.62	217.41	217.21	217.01	216.77	216.89	
	$\frac{1}{4}$	217.55	217.56	217.33	217.12	216.92	216.72	216.48	216.60	
14	0	217.28	217.29	217.05	216.84	216.64	216.44	216.20	216.32	
	$\frac{1}{4}$	216.98	216.99	216.76	216.55	216.35	216.15	215.91	216.03	
	$\frac{1}{2}$	216.68	216.69	216.46	216.25	216.05	215.85	215.61	215.73	
	$\frac{3}{4}$	216.38	216.39	216.16	215.95	215.75	215.55	215.31	215.43	
	1	216.11	216.12	215.88	215.67	215.47	215.27	215.03	215.15	
	$\frac{1}{4}$	215.81	215.82	215.59	215.38	215.18	214.98	214.74	214.86	
15	0	215.54	215.55	215.31	215.10	214.90	214.70	214.46	214.58	
	$\frac{1}{4}$	215.24	215.25	215.02	214.81	214.61	214.41	214.17	214.29	
	$\frac{1}{2}$	214.94	214.95	214.71	214.50	214.30	214.10	213.86	213.98	
	$\frac{3}{4}$	214.64	214.65	214.42	214.21	214.01	213.81	213.57	213.69	
	1	214.37	214.38	214.14	213.93	213.73	213.53	213.29	213.41	
	$\frac{1}{4}$	214.07	214.08	213.84	213.63	213.43	213.23	212.99	213.11	
16	0	213.80	213.81	213.57	213.36	213.16	212.96	212.72	212.84	
	$\frac{1}{4}$	213.50	213.51	213.27	213.06	212.86	212.66	212.42	212.54	
	$\frac{1}{2}$	213.20	213.21	212.97	212.76	212.56	212.36	212.12	212.24	
	$\frac{3}{4}$	212.90	212.91	212.67	212.46	212.26	212.06	211.82	211.94	
	1	212.63	212.64	212.39	212.18	211.98	211.78	211.54	211.66	
	$\frac{1}{4}$	212.33	212.34	212.10	211.89	211.69	211.49	211.25	211.37	
17	0	212.06	212.07	211.82	211.61	211.41	211.21	210.97	211.09	
	$\frac{1}{4}$	211.76	211.77	211.52	211.31	211.11	210.91	210.67	210.79	
	$\frac{1}{2}$	211.46	211.47	211.22	211.01	210.81	210.61	210.37	210.49	
	$\frac{3}{4}$	211.16	211.17	210.93	210.72	210.52	210.32	210.08	210.20	
	1	210.89	210.90	210.65	210.44	210.24	210.04	209.80	209.92	
	$\frac{1}{4}$	210.59	210.60	210.36	210.15	209.95	209.75	209.51	209.63	

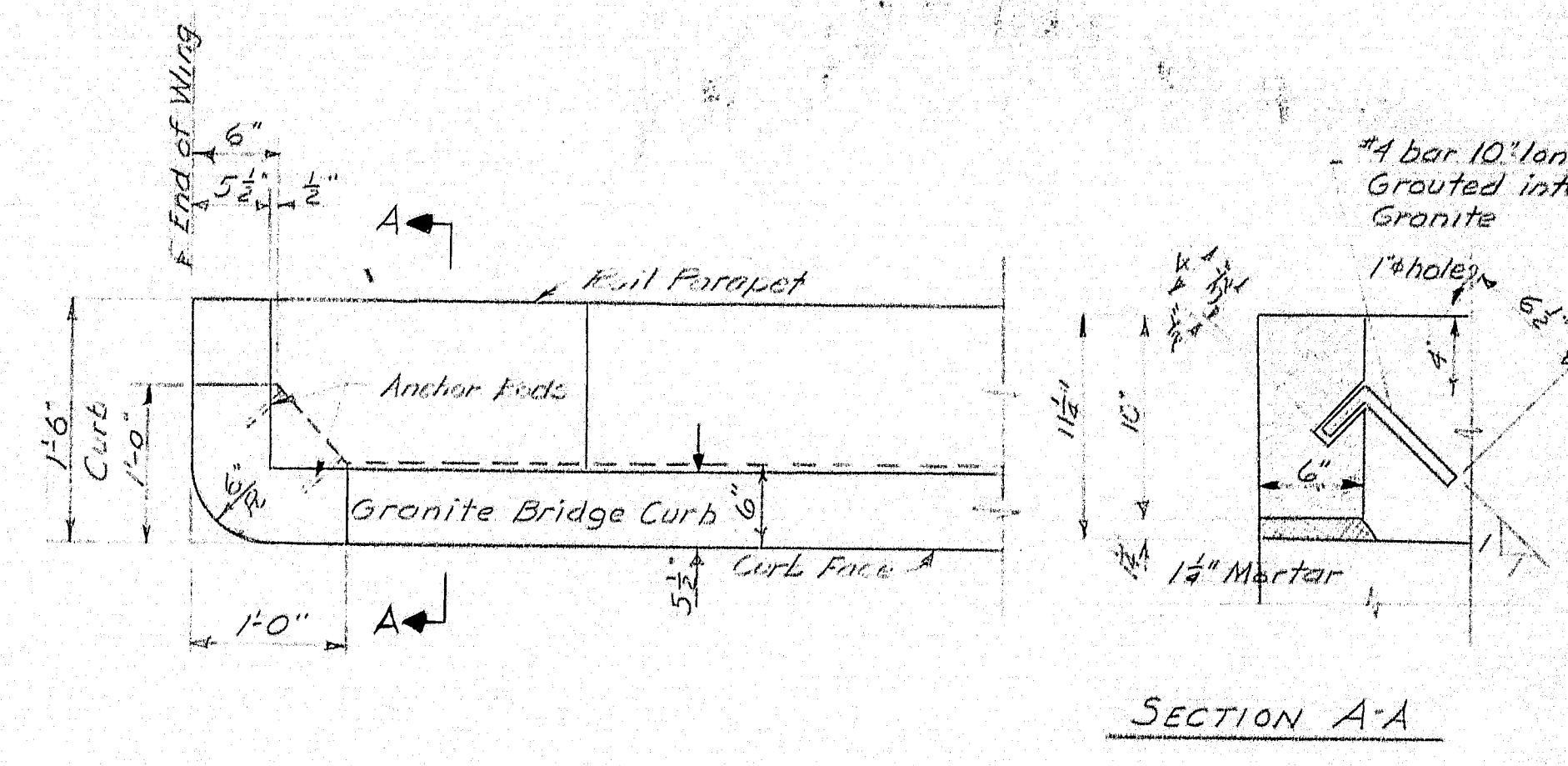
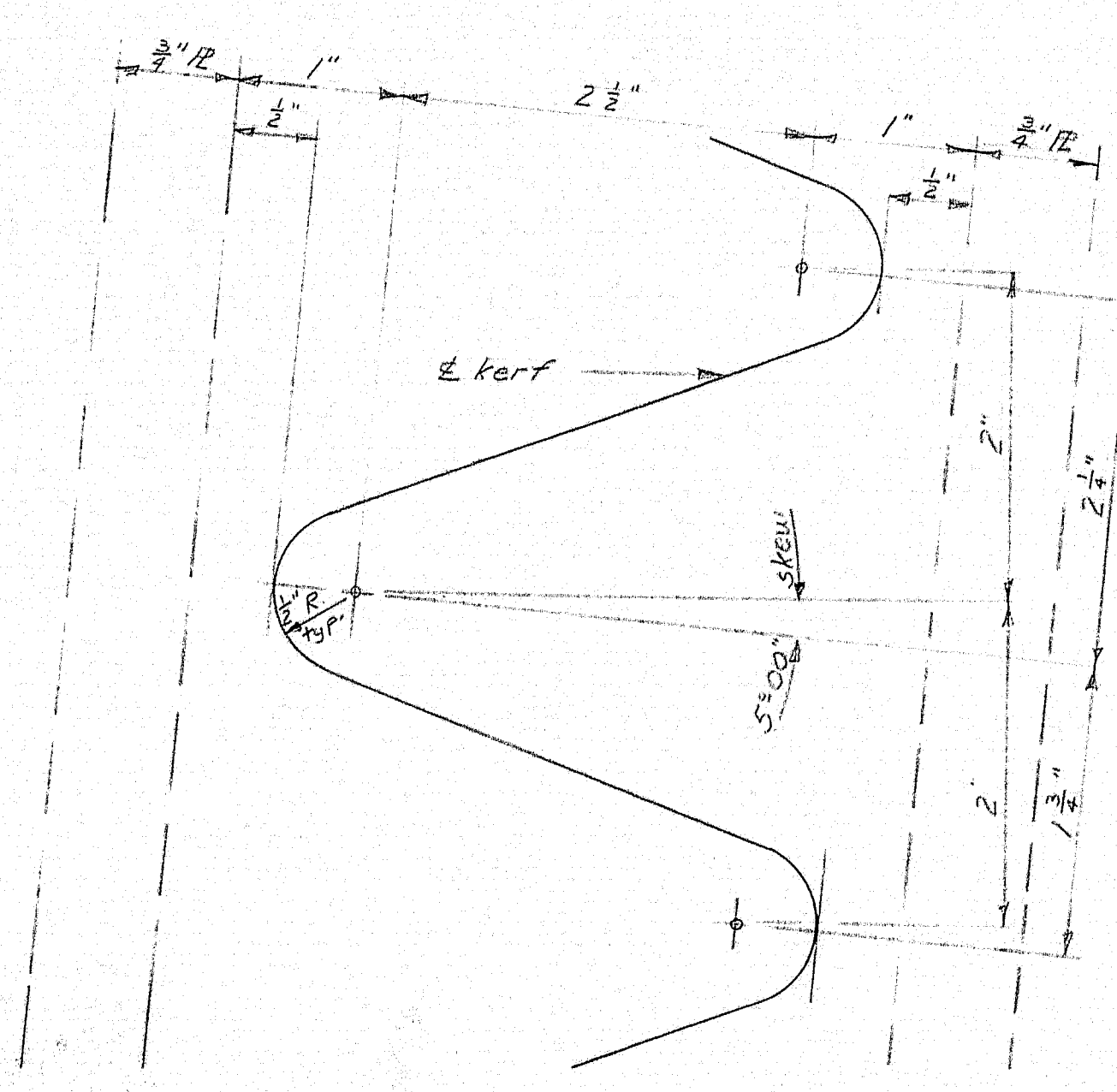
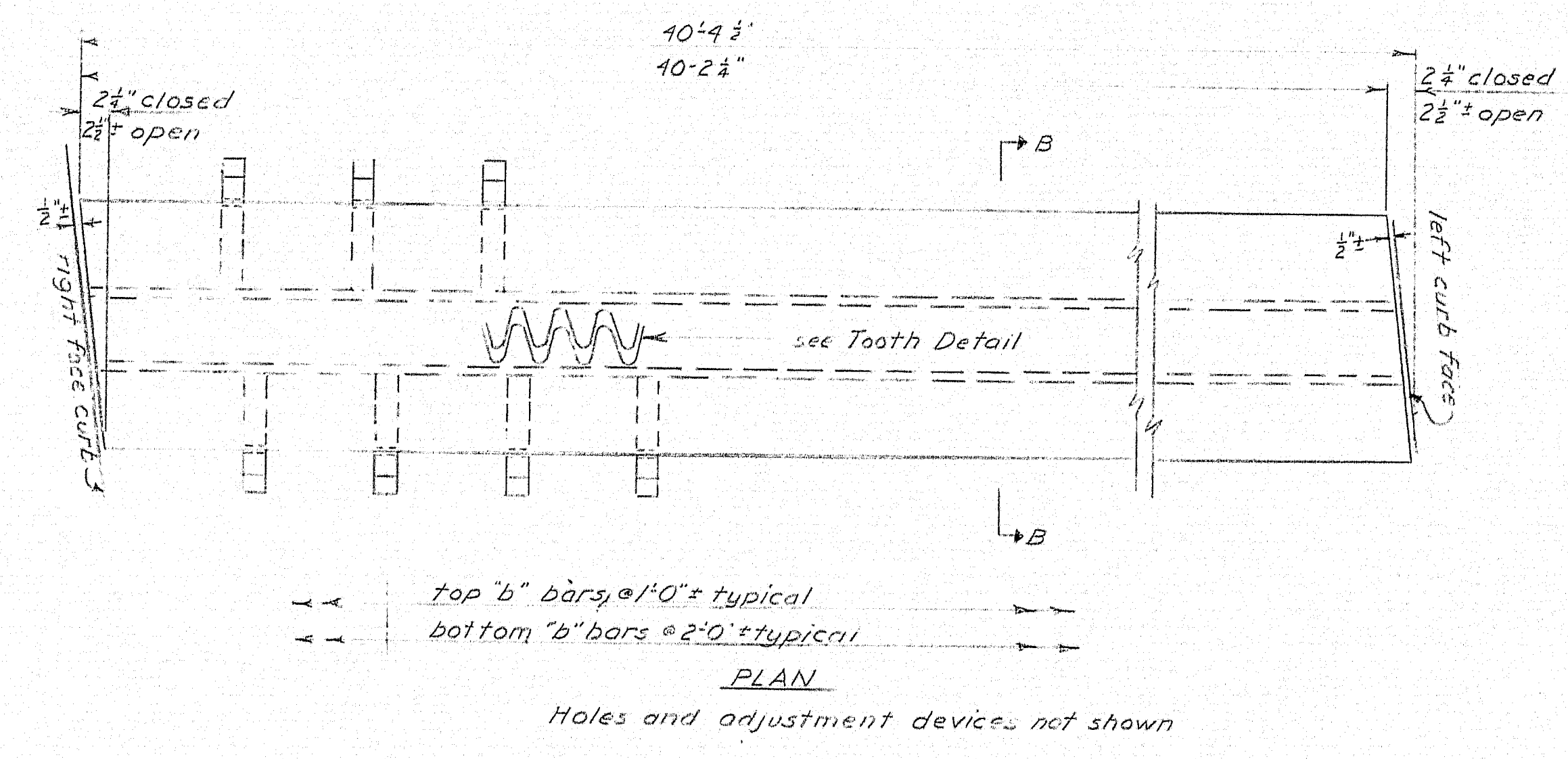


ARMORED JOINT DETAILS

Notes
Two each required as shown: 1 for S.B. lane, 1 for N.B. lane.
No allowance for slope or vertical curvature is necessary in fabrication.
Armored joints may be fabricated in halves lengthwise and welded in the field in a manner approved by the Engineer.
Adjustment device plates shall be approximately centered on stringers.
Weld adjustment device together after armored joint is in position.

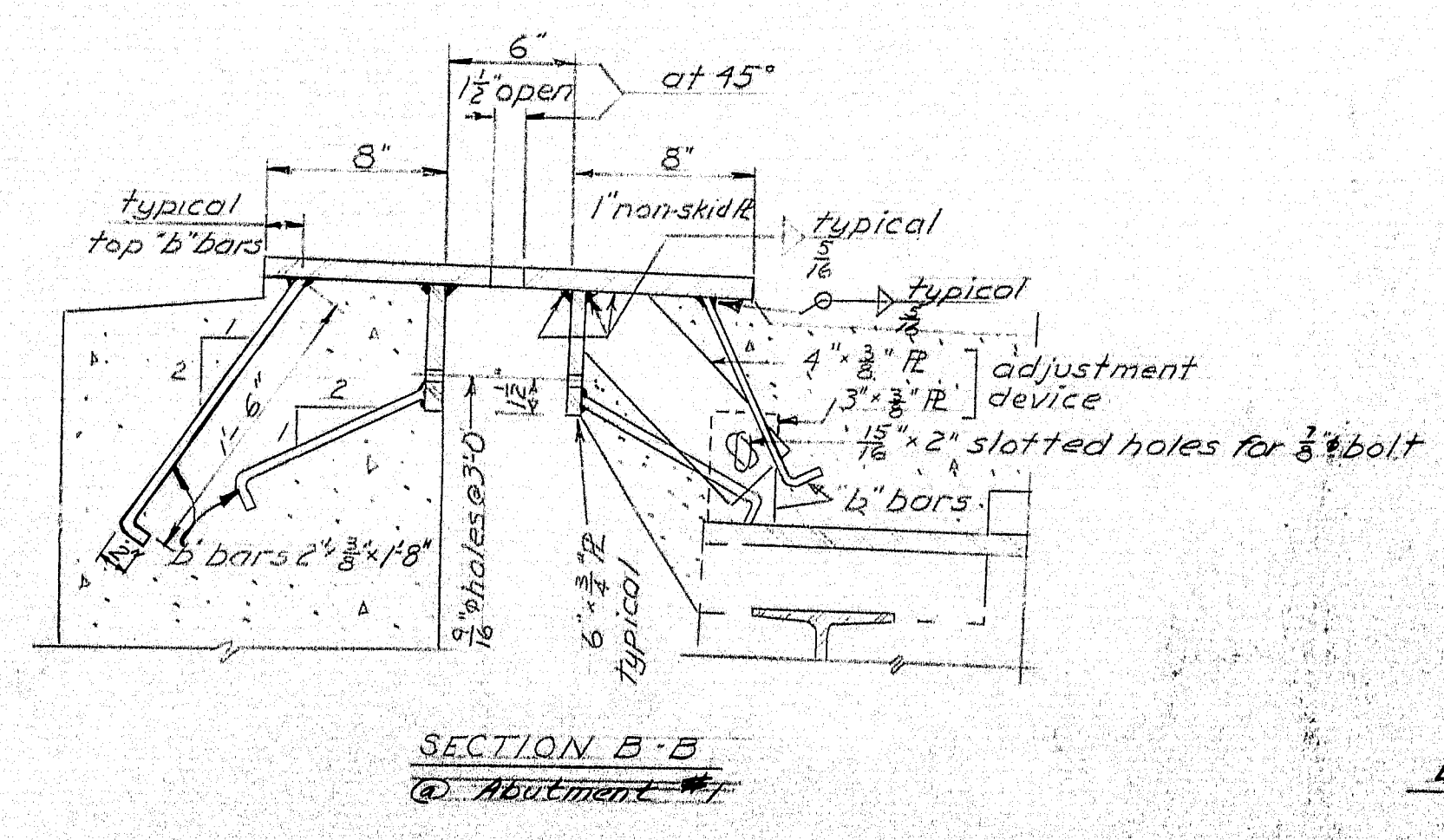
APPROACH SLAB DETAILS

Approach slab concrete to be class A and to be paid for under Item 701-34 Portland Cement Concrete, Abutments and Retaining Walls.



GRANITE BRIDGE CURB DETAIL AT ABUTMENT WINGS

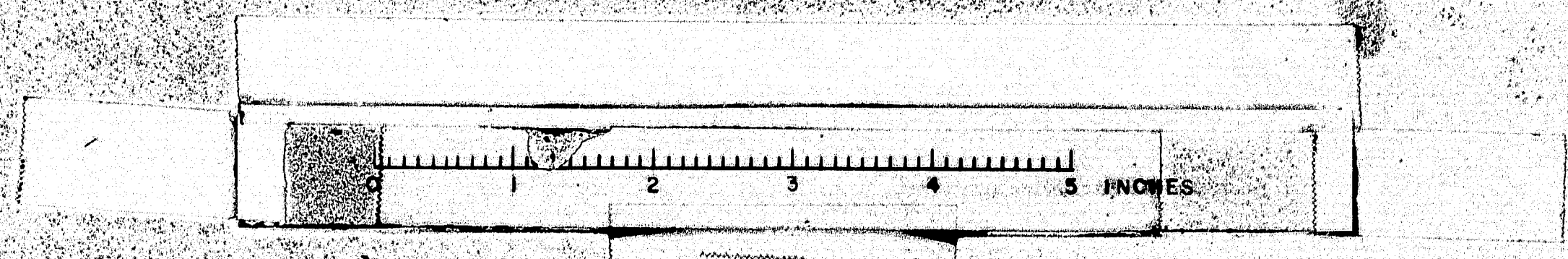
Note: #4 bar 10" long not included in Reinf. Steel Schedule

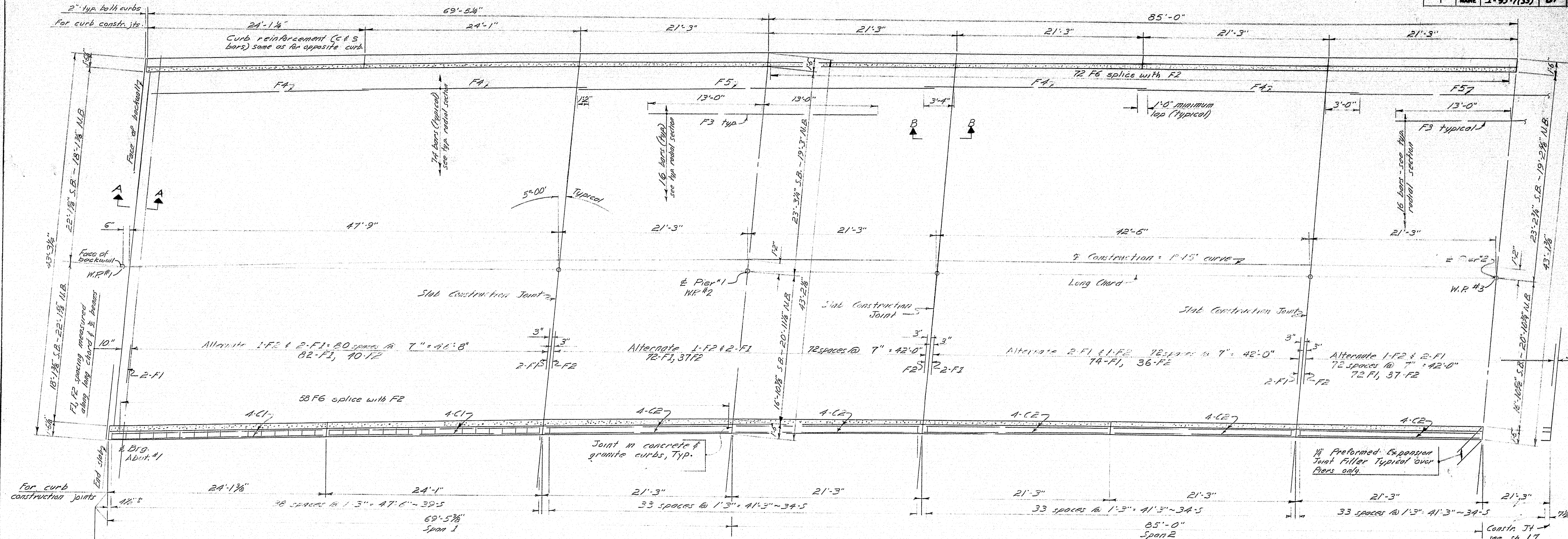


Notes: Similar to notes for armored joints

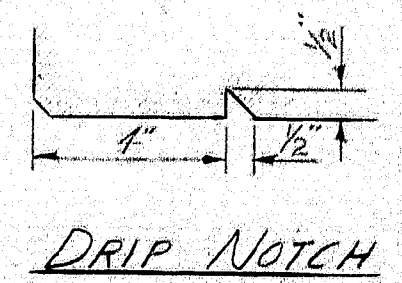
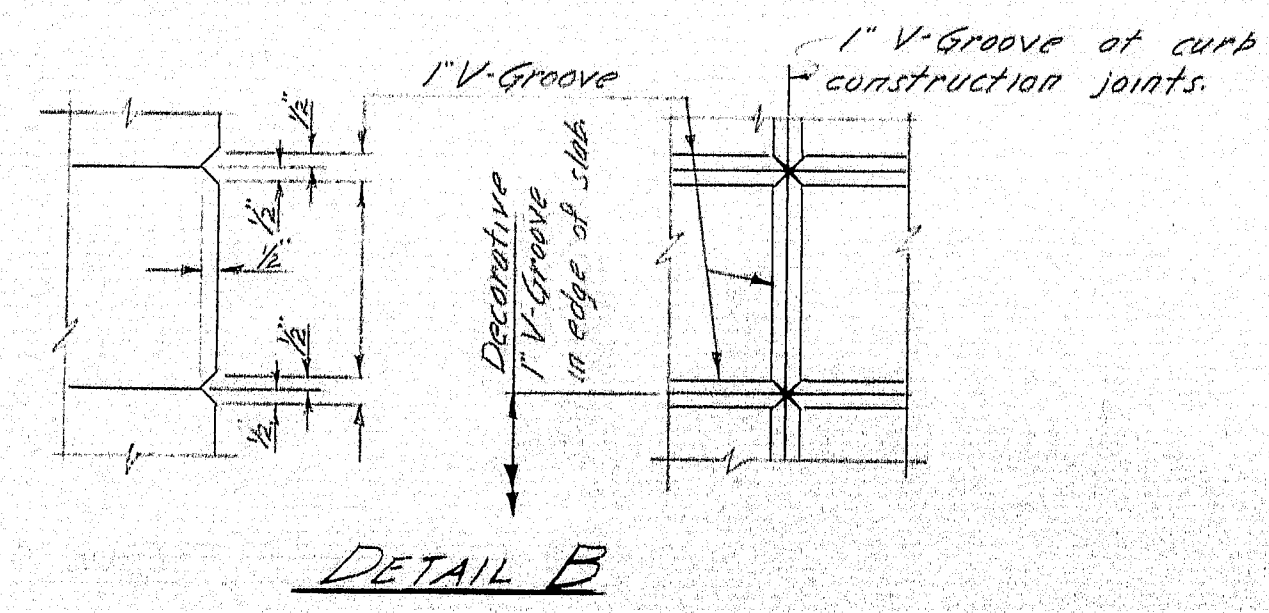
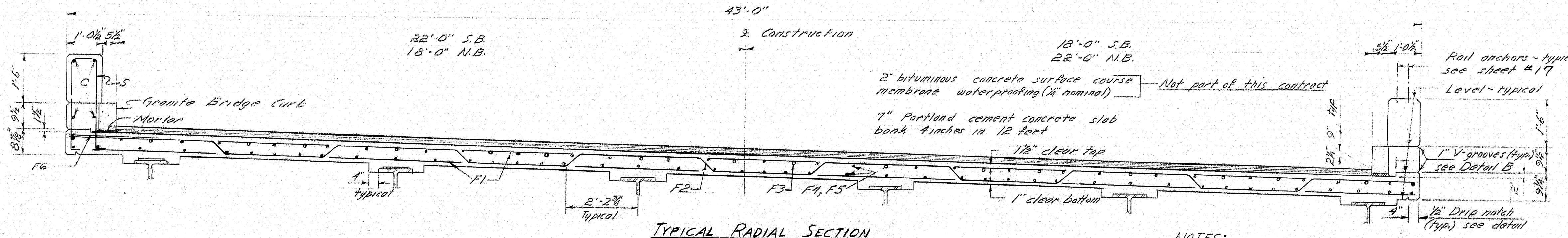
EXPANSION DAM DETAILS

DESIGN	T.M.K. & R.D.
CHECK	2/6/65
STATE HIGHWAY COMMISSION BRIDGE DIVISION	
SEBASTICOOK RIVER BRIDGE	
IN THE TOWN OF PITTSFIELD	
SOMERSET COUNTY	
ARMORED JOINT AND EXPANSION DAMS	
SHEET 15 OF 18	AUGUSTA, MAINE FEB. 1963





Area dimensioned along fascia and are equal to their chord lengths. Curved C&S are along the equal lengths of adjacent fascia areas. Dimensions horizontal (typical).

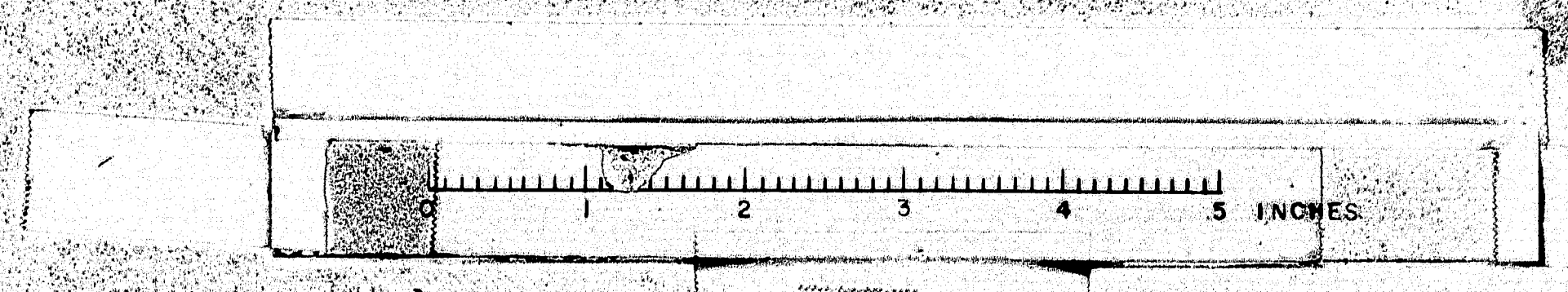


NOTES:

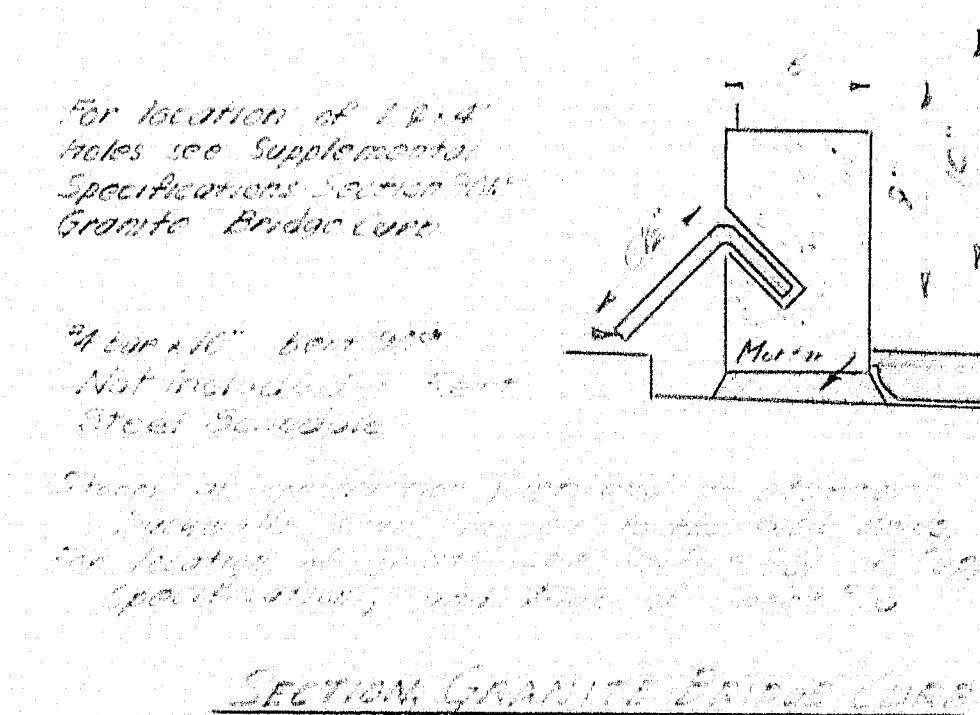
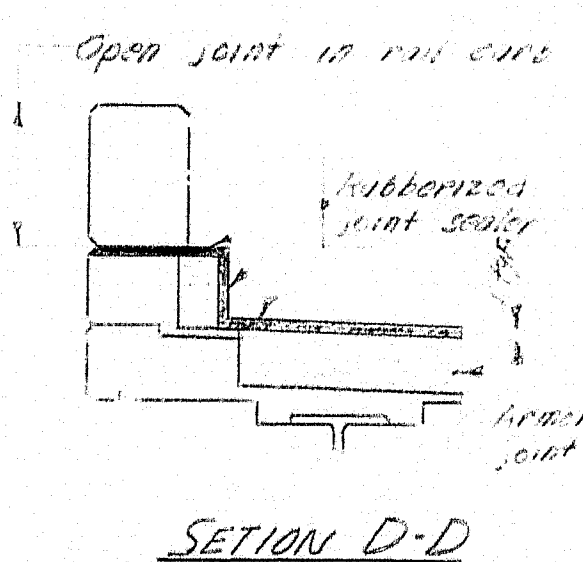
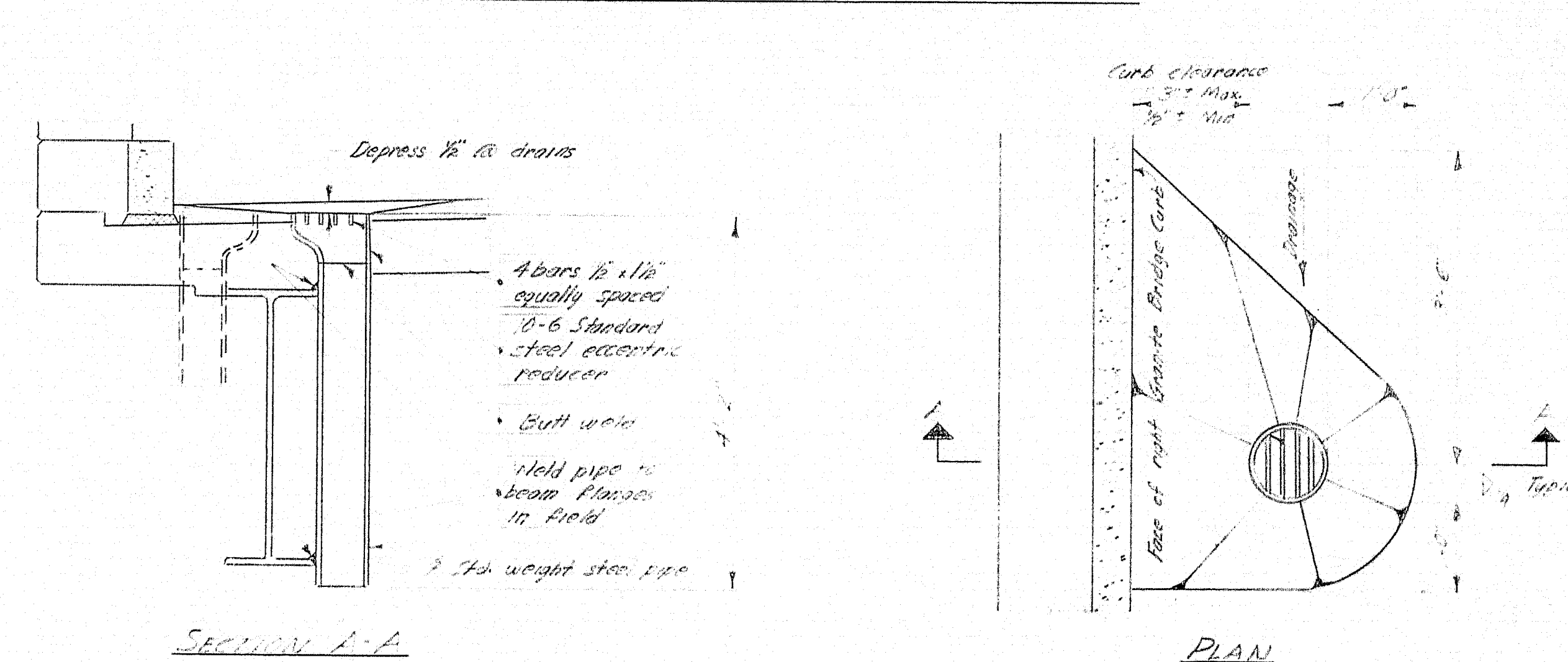
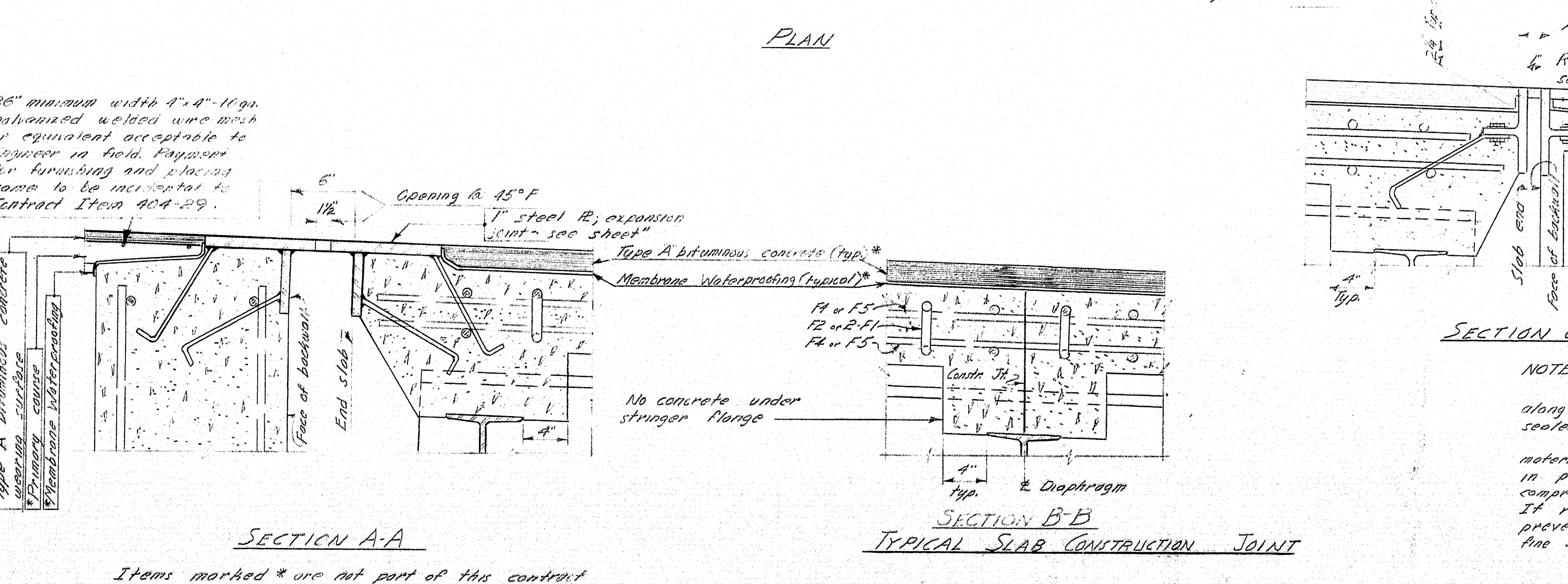
- Reinforcing**-Steel for curbs shall be in place before slab concrete is placed, and that for rail parapets before curb concrete is placed.
- Concrete Placement**-For slab sequence, see sheet #17. Concrete for curbs shall not be placed until slab concrete has been in place 7 days. During the 7 days forms may be constructed, but only hand equipment will be allowed on slab.
- Chamfer**-All exposed edges of concrete 1/2".
- Rail**-For plan see sheet #17.
- Joints**-For joints at ends of slab see Sections A-A & C-C, sheet #17. At all other (i.e., intermediate) joints in slab and concrete curbs, break the bond between concrete surfaces with a coat of asphalt paint.
- Blacking**-See sheet #17.

Work this sheet with sheet #17

DESIGN T.H.K. DETAIL E.D. CHECK A.R.S.	TRACE R.T.A.
STATE HIGHWAY COMMISSION BRIDGE DIVISION	
SEBASTICOOK RIVER BRIDGE	
IN THE TOWN OF PITTSFIELD	
SOMERSET COUNTY	
SUPERSTRUCTURE NO. 130	
SHEET 16 OF 18 AUGUSTA, MAINE FEB 1963	



B. P. R. REG. NO.	STATE	PROJECT NUMBER	SHEET NO.	TOTAL SHEET
1	MAINE	I-95-7(35)	62	225



NOTE:

Opening between curb and backwall of Abutment "E" is to be sealed along the top of the slab, along the roadway, with rubberized joint sealer.

Rubberized joint sealer shall be supported on non bituminous material. At the contractor's option the supporting material may be left in place or removed. If left in place, said material shall be compressible in accordance with A.A.S.H.O. specification M153-54. If removed, bonding with the rubberized joint sealer shall be prevented by a method satisfactory to the Engineer (layer of the sand, wax paper, etc.)

DESIGN *T.H.K.* DETAIL *R.D.*
TRACE *R.T.A.*
CHECK *A.E.S.*

STATE HIGHWAY COMMISSION
BRIDGE DIVISION

SEBASTICOOK RIVER BRIDGE

IN THE TOWN OF
PITTSFIELD
SOMERSET COUNTY

SUPERSTRUCTURE *N.B. & S.B.*

SHEET 17 OF 18 AUGUSTA, MAINE FEB. 1963

M-1997

